

XVI ECSMGE 2015

13-17 September 2015 - Edinburgh



Effectiveness of Remedial Measures Applied to Mitigate Differential Movement at Railroad Track Transitions

Erol Tutumluer, Professor of Civil Engineering
Paul F. Kent Endowed Faculty Scholar
tutumlue@illinois.edu

University of Illinois at Urbana-Champaign

TC202 Workshop on Railroad Geotechnics, September 13, 2015



ILLINOIS
UNIVERSITY OF ILLINOIS AT URBANA-CHAMPAIGN





Federal Railroad Administration High Speed Rail Broad Agency Announcement (BAA) Project

Mitigation of Differential Movement at Railway Transitions for US High Speed Passenger Rail and Joint Passenger/Freight Corridors

PI: Erol Tutumluer

Co-PI: Timothy D. Stark

Research Engineer: Deb Mishra

Visiting Research Scholar: James P. Hyslip



RAILTEC



BUILDING AMERICA®



Research Objective

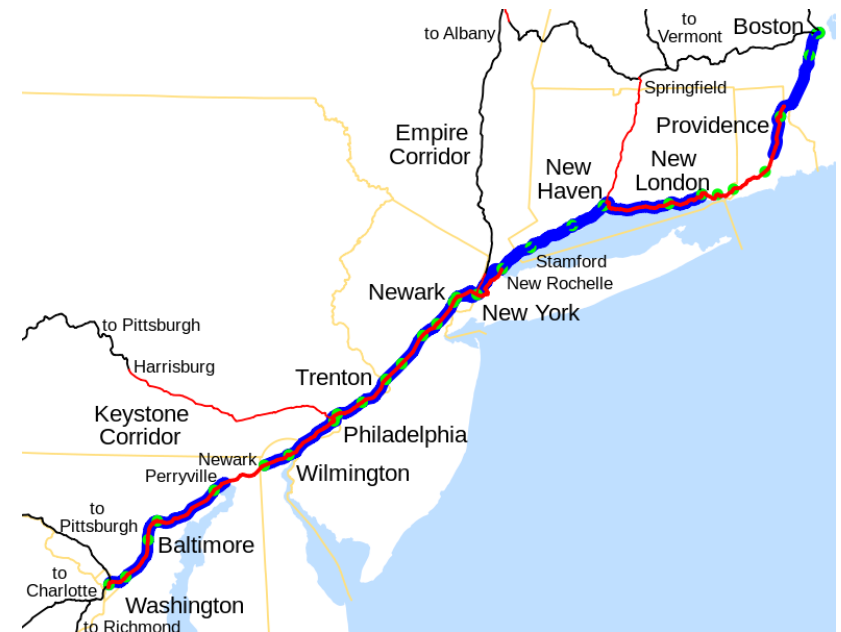
Development of design and repair techniques to minimize and mitigate, respectively, **differential movement at railway transitions** to **ensure safe high speed operation**

Project Duration: 51 Months (Oct. 2011 – Dec. 2015)

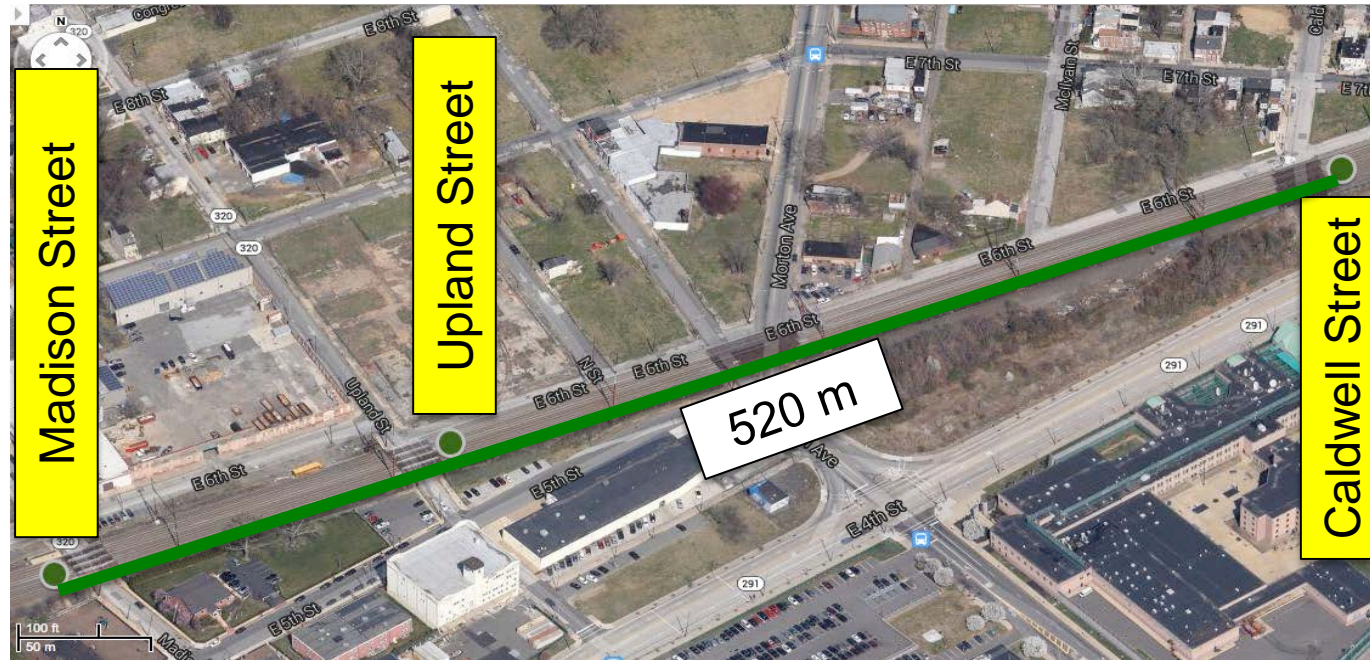
- Scope
1. ***Monitor Railway Transitions for Differential Movement***
 2. ***Identify "Location" and Major Factors Causing Differential Movement at Monitored Railroad Transitions***
 3. ***Numerical Modeling of Monitored Railway Transitions and of Preferred Design and Rehabilitation Techniques for Rail Transitions***

AMTRAK Northeast Corridor (NEC)

- ✓ The NEC is 457 miles long & has:
 - ✓ 17 tunnels
 - ✓ 1,186 bridges
- ✓ 2,220 passenger trains daily
- ✓ 70 freight trains daily (over 14 million car-miles of freight per year)
- ✓ 720,000 people ride along some part of the corridor each day.
- ✓ In 2011, 11 million passengers on Amtrak's Northeast Regional and Acela Express services



Three Bridge Approaches at Chester, PA Site



**AMTRAK
NEC**

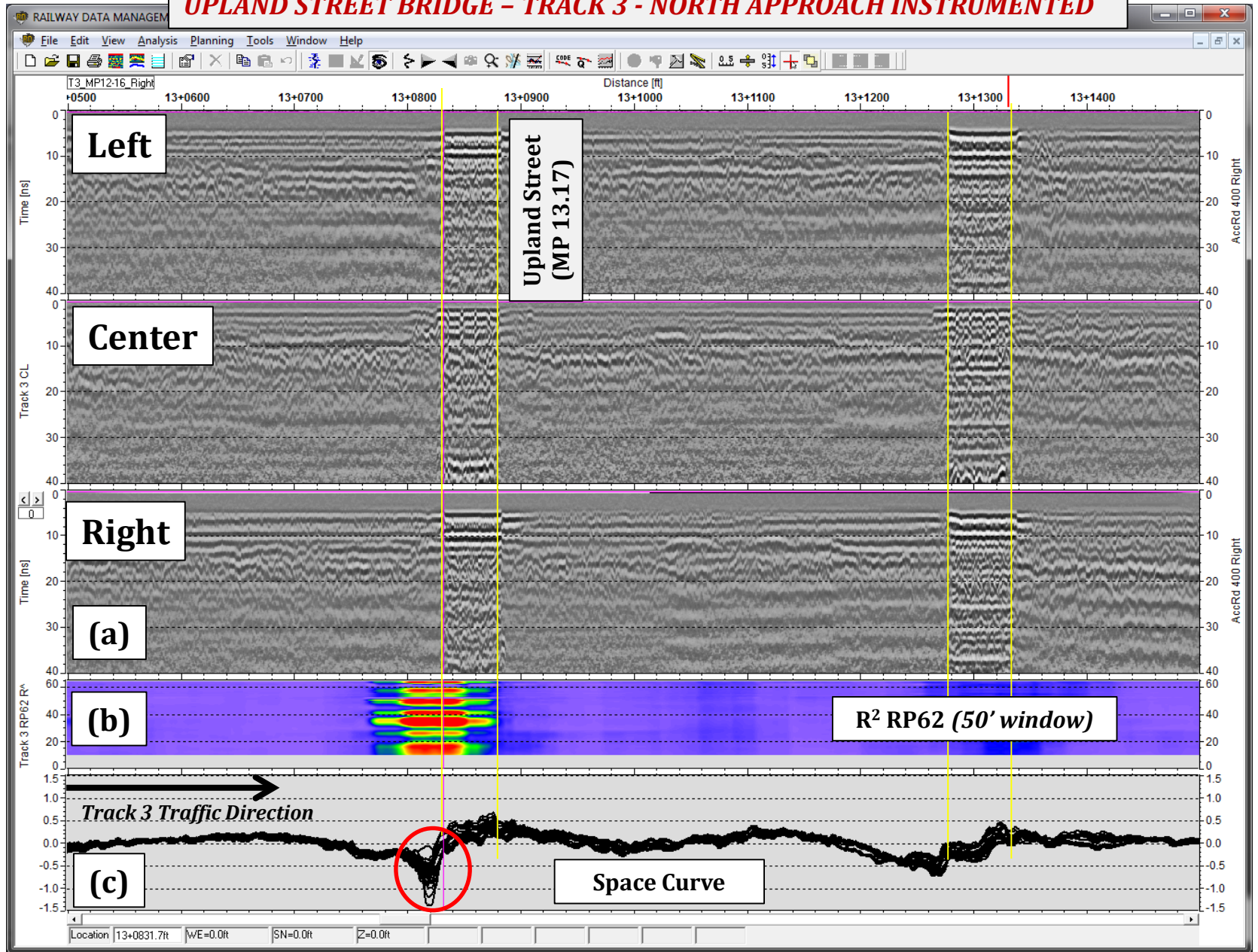
**ACELA
Passenger
Trains
110 mph
(177 km/h)**

**Recurring bridge approach
settlement and geometry problems**

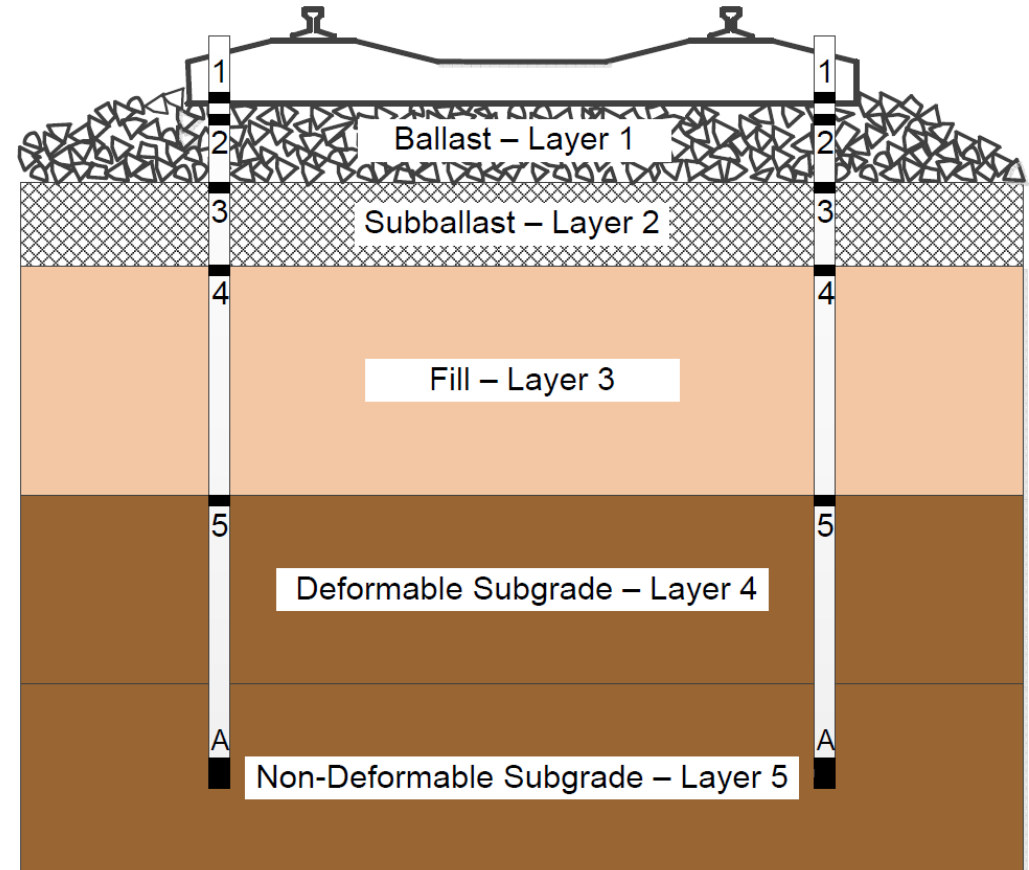
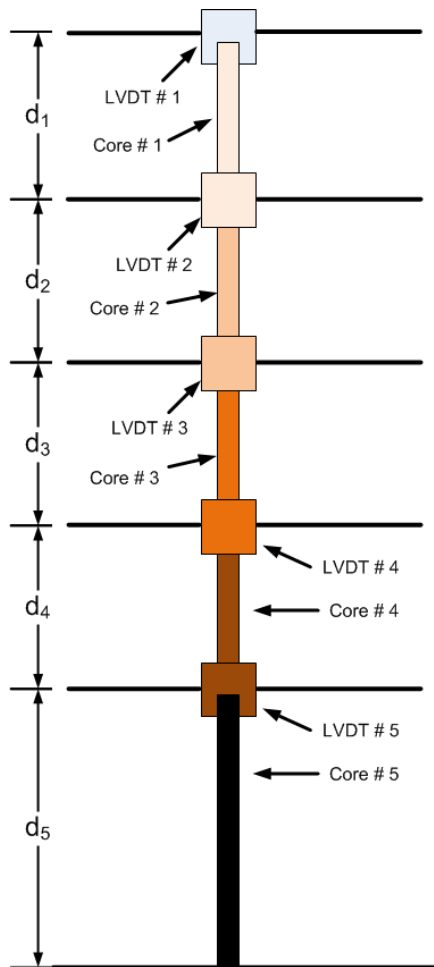
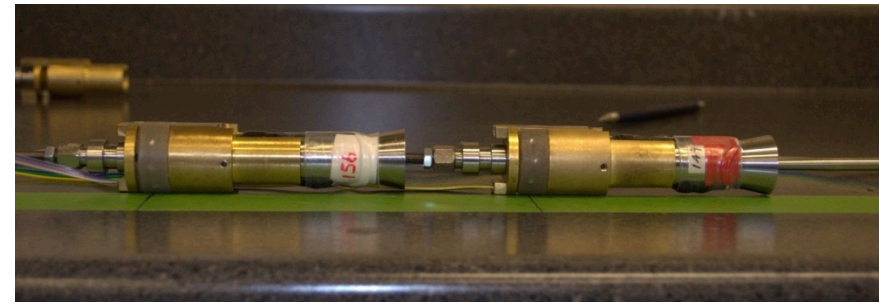
**Chester Amtrak Station
Bridge Approach (January 2012)**



UPLAND STREET BRIDGE - TRACK 3 - NORTH APPROACH INSTRUMENTED



Installed **Multidepth** **Deflectometers (MDDs)** at 3 Bridge Approaches



The Independent Anchoring MDD System

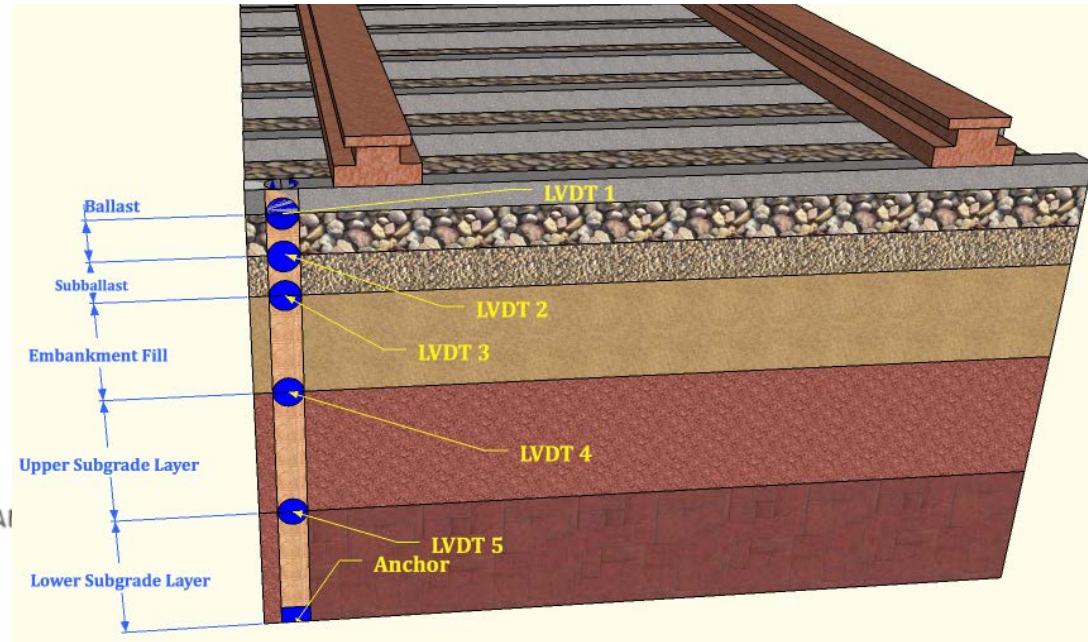
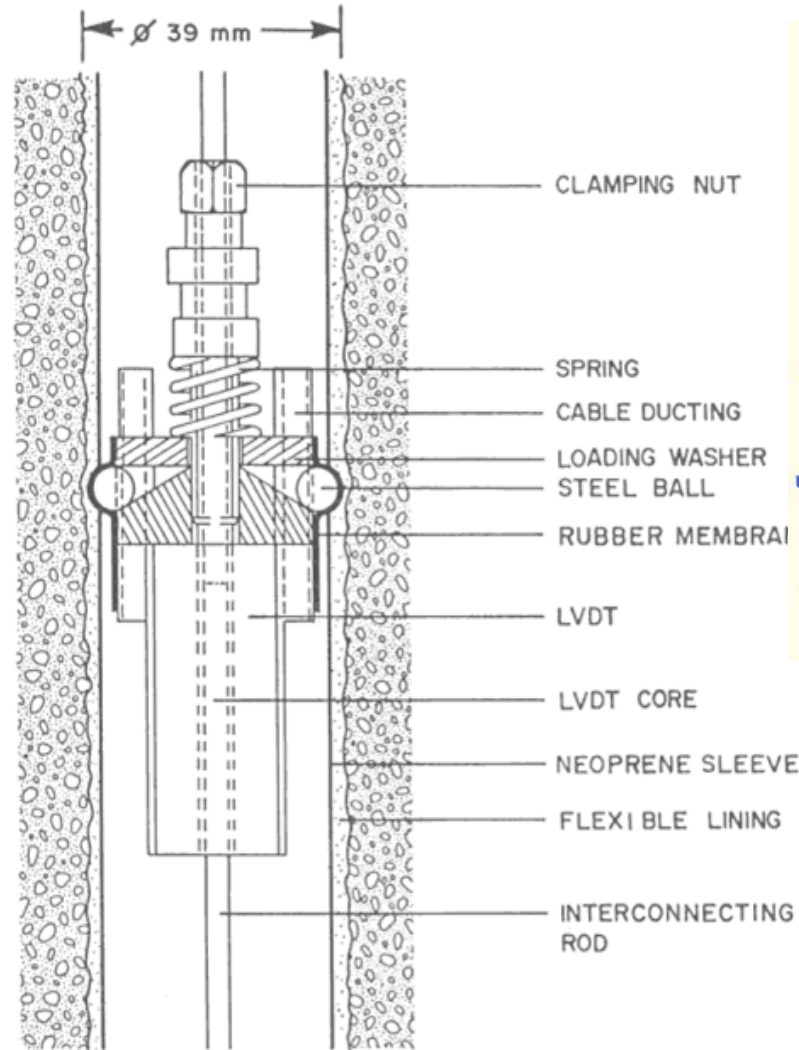


Figure Source: DeBeer, 1989

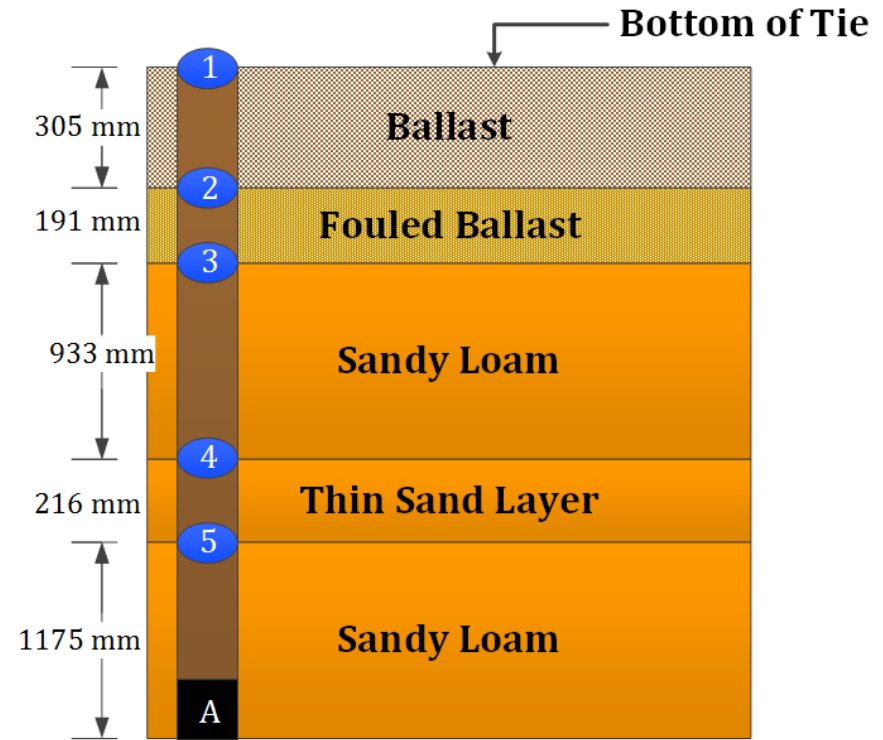
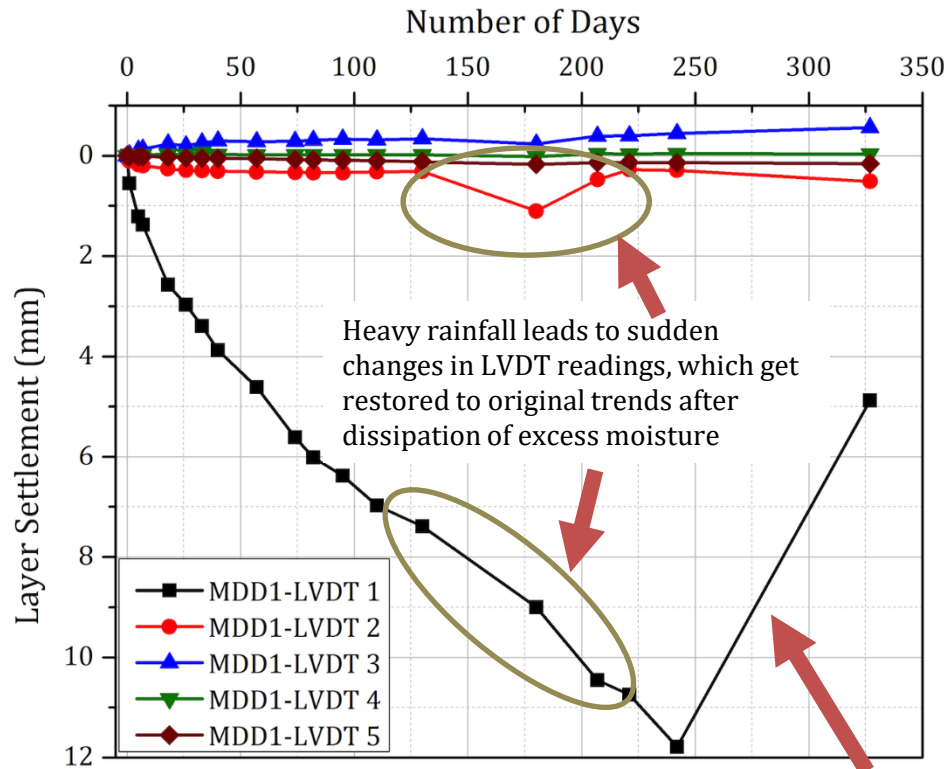
Multidepth Deflectometer Module



Assembled MDD Module

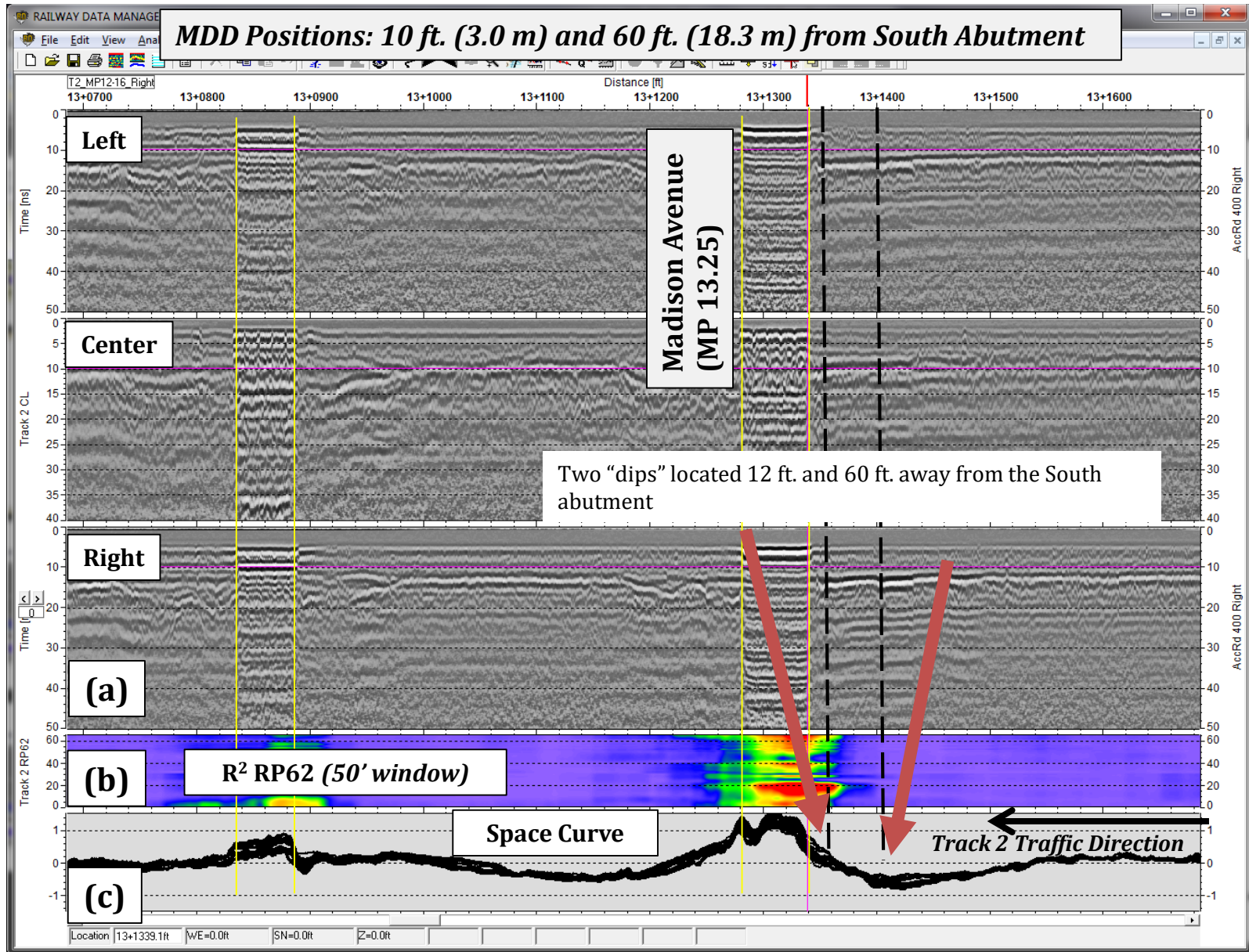
Layer Settlements – Upland St.

15 ft. from North Abutment



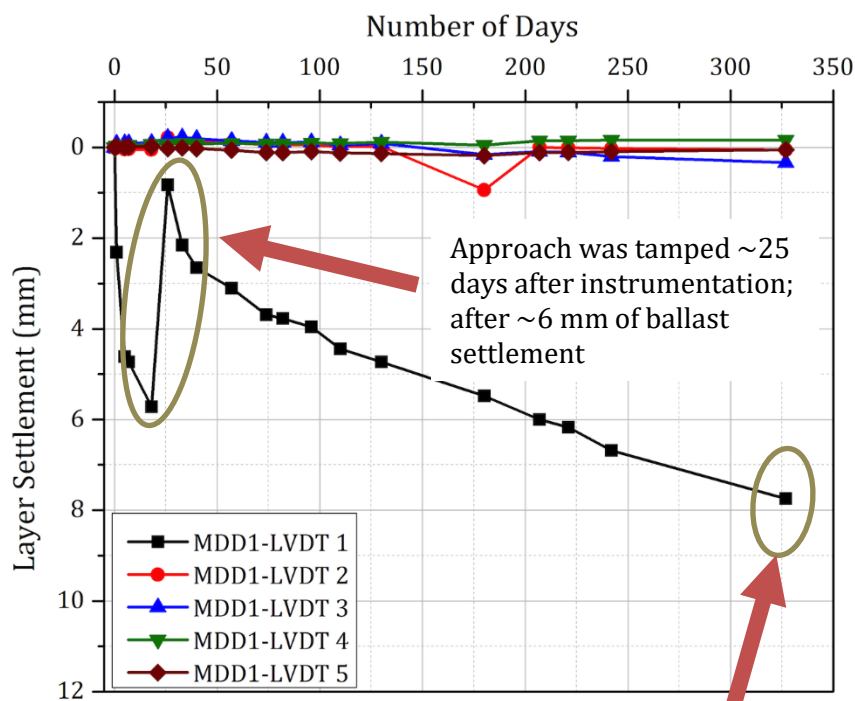
Up to 12 mm of ballast settlement recorded before track resurfacing

Madison Street Bridge-South Approach-Track 2

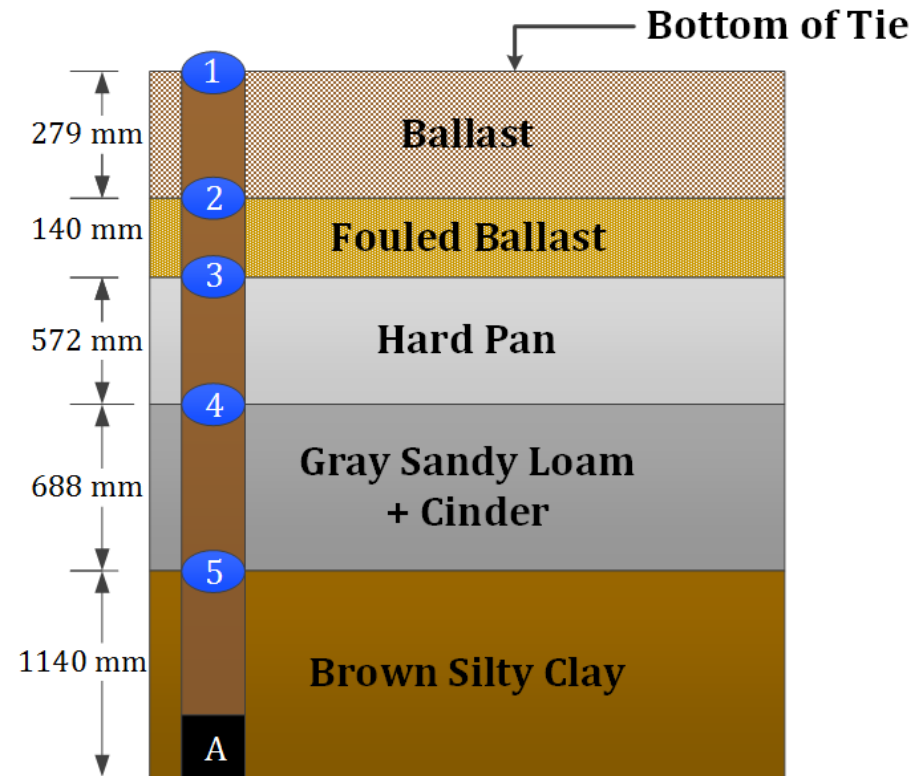


Layer Settlements – Madison St.

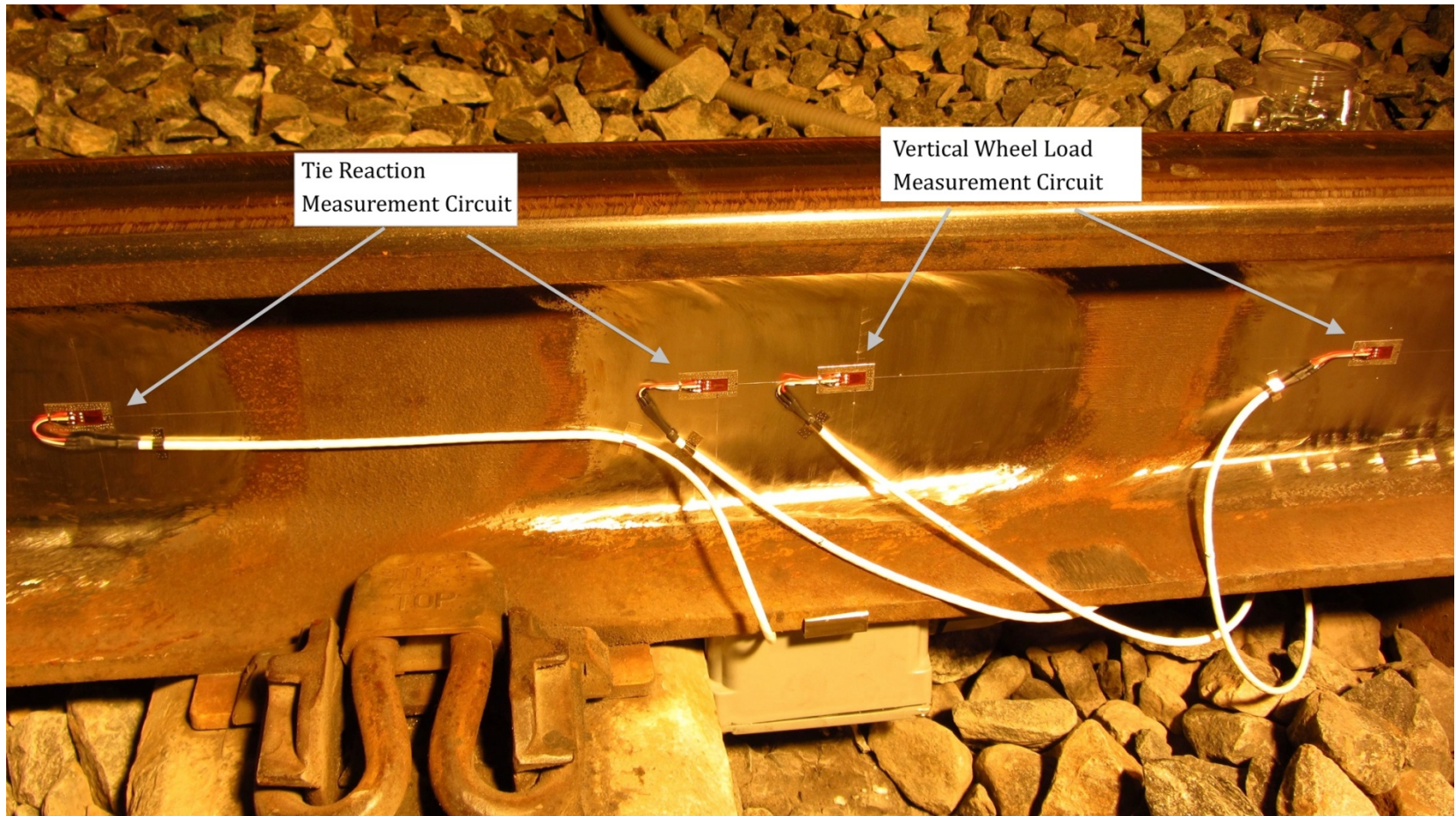
12 ft. from South Abutment



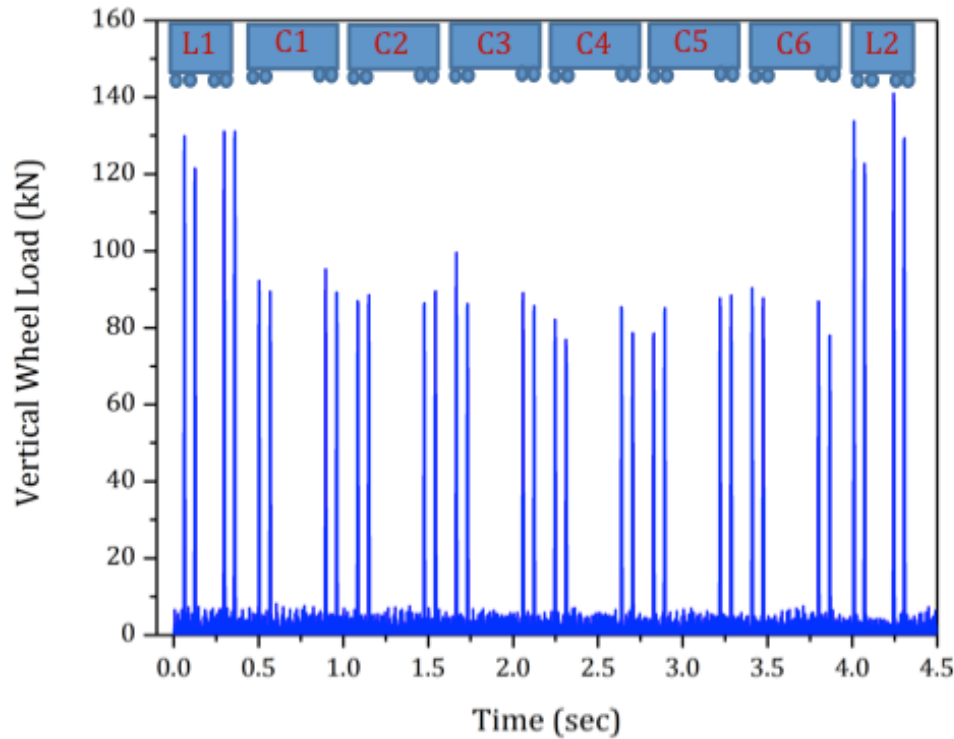
Last set of measurement indicated a total ballast settlement of ~ 8 mm. Due for tamping ?



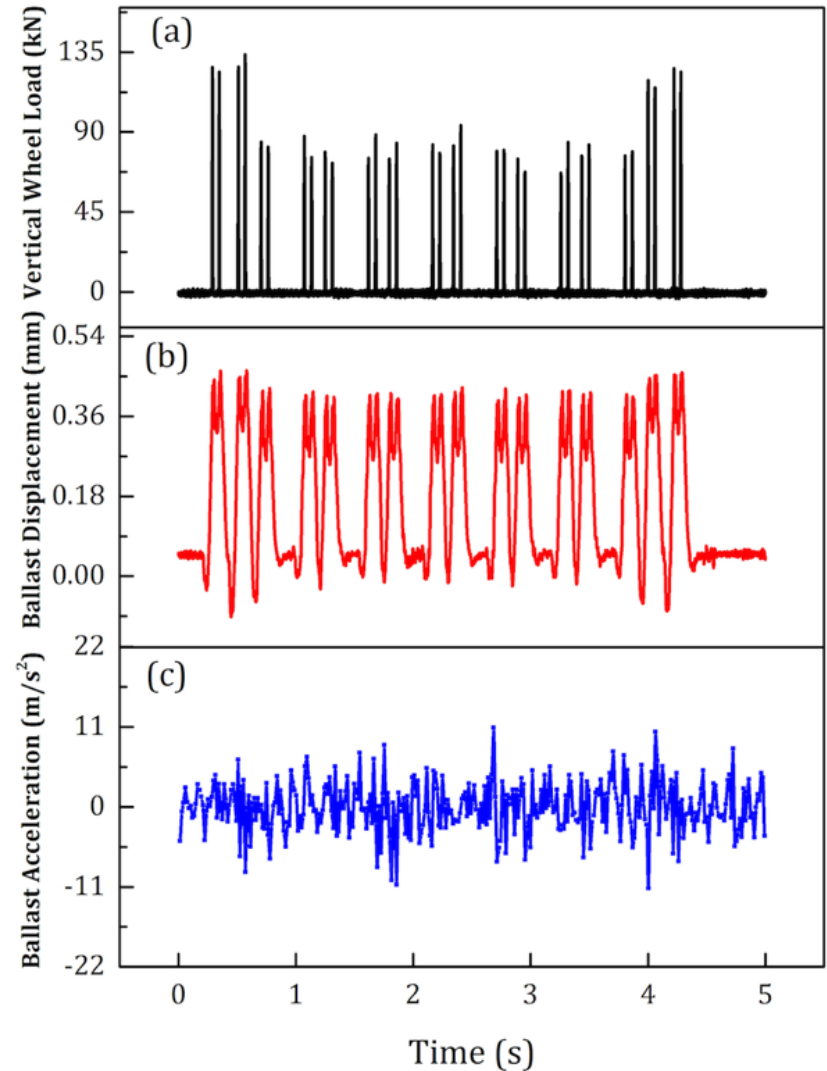
Strain Gauge Layout – Wheel Loads



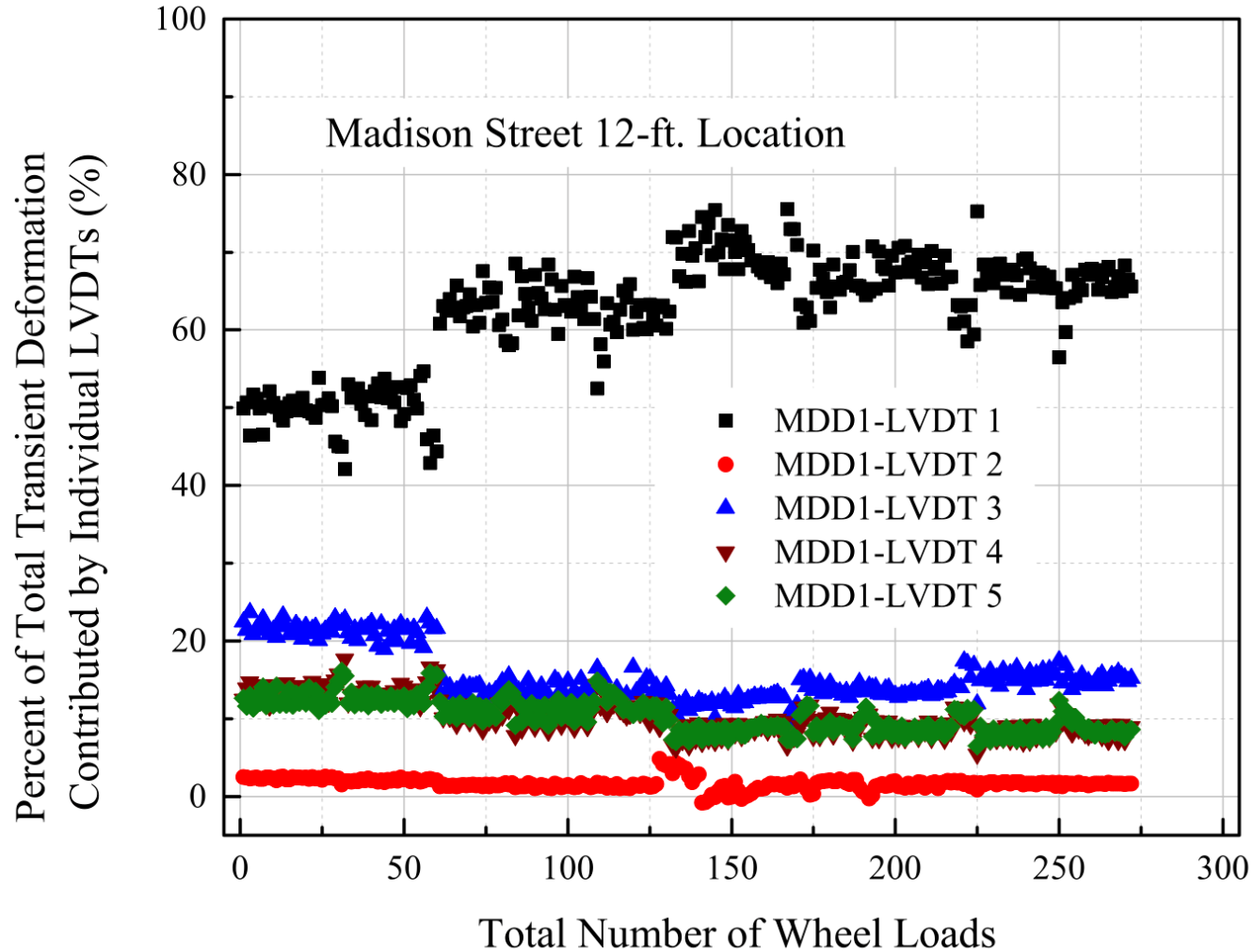
Measured (a) Wheel Loads; (b) Ballast Layer Displacements; and (c) Ballast Accelerations



Acela Express
Wheel Loads



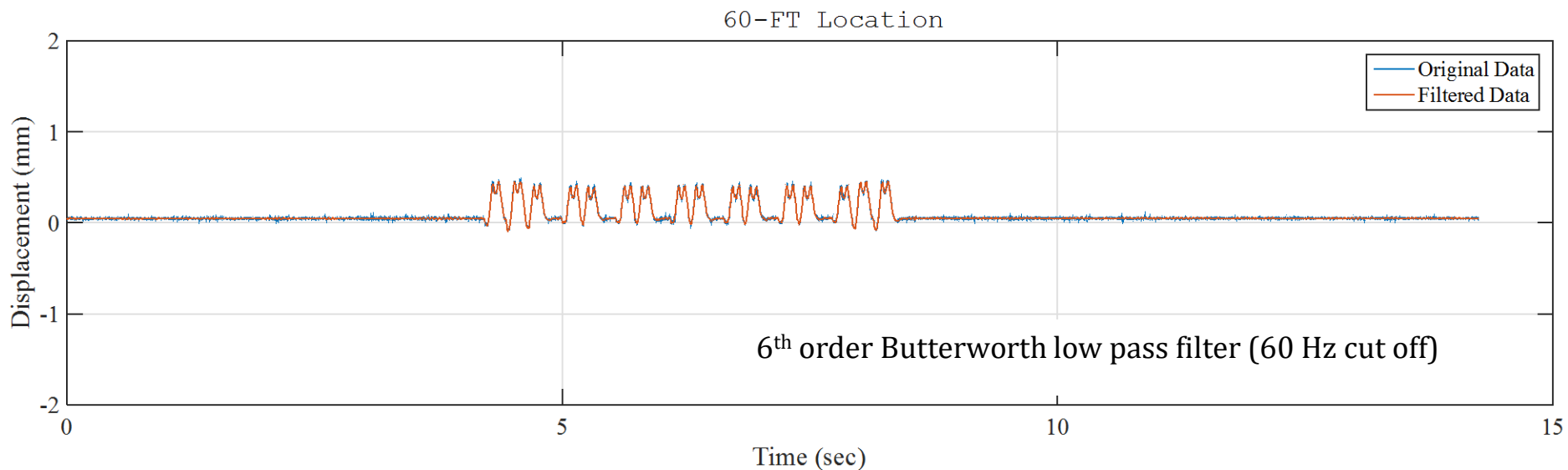
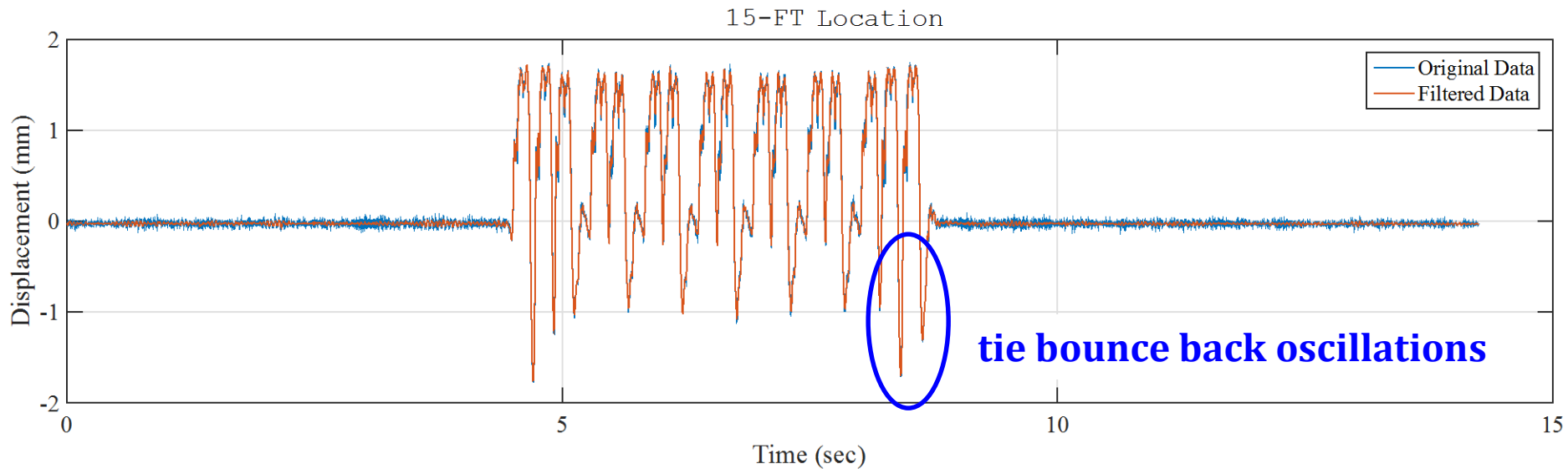
Summary Across All Months



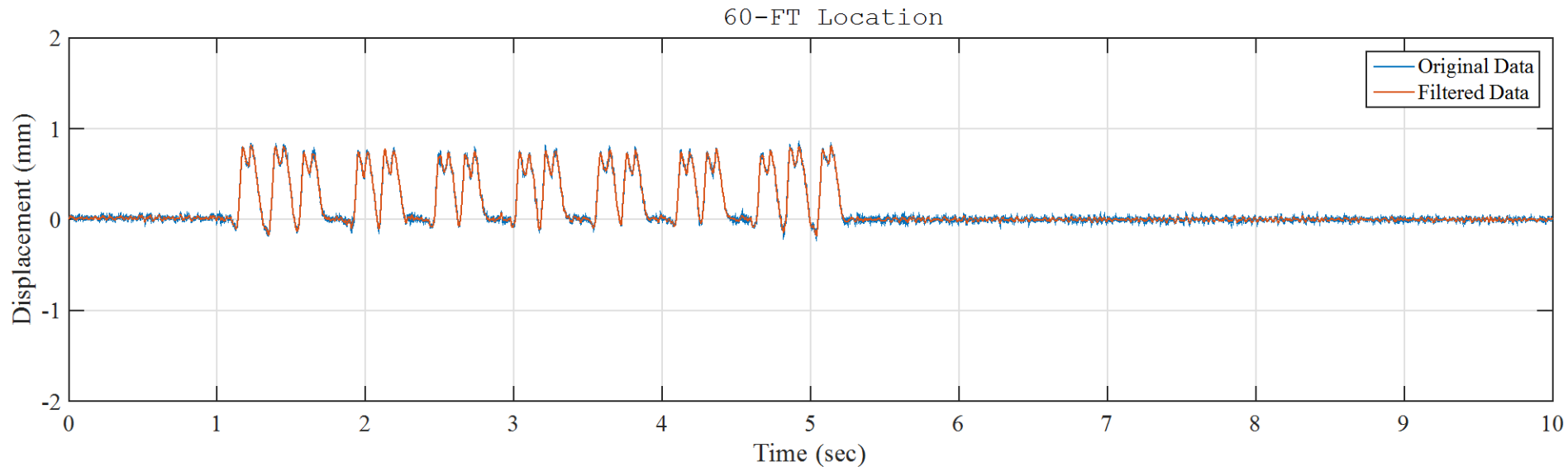
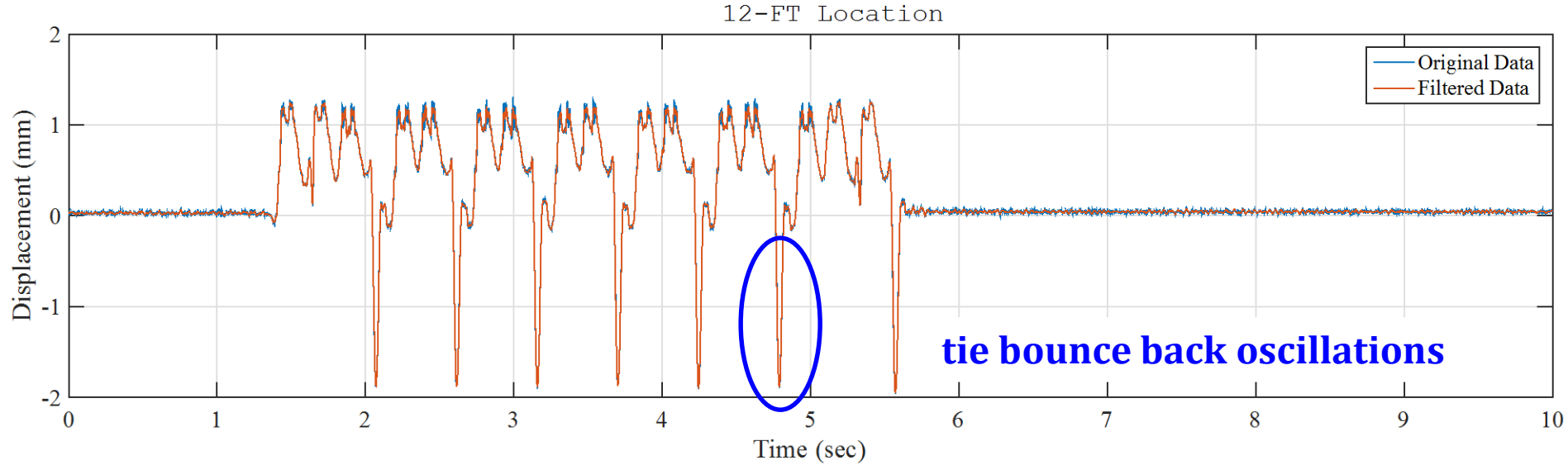
LVDT 1 accounts for 40-75% of the total transient deformations

August 2012 – LVDT 1 Data

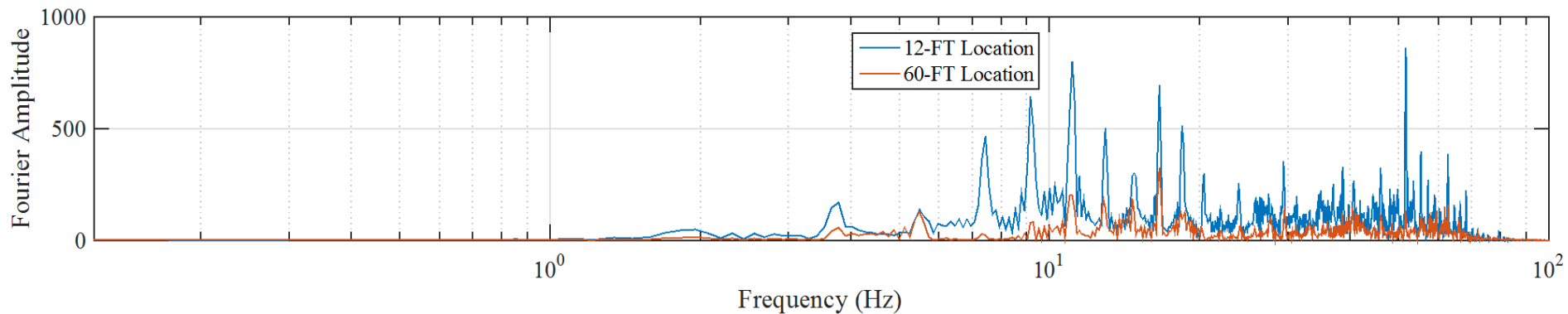
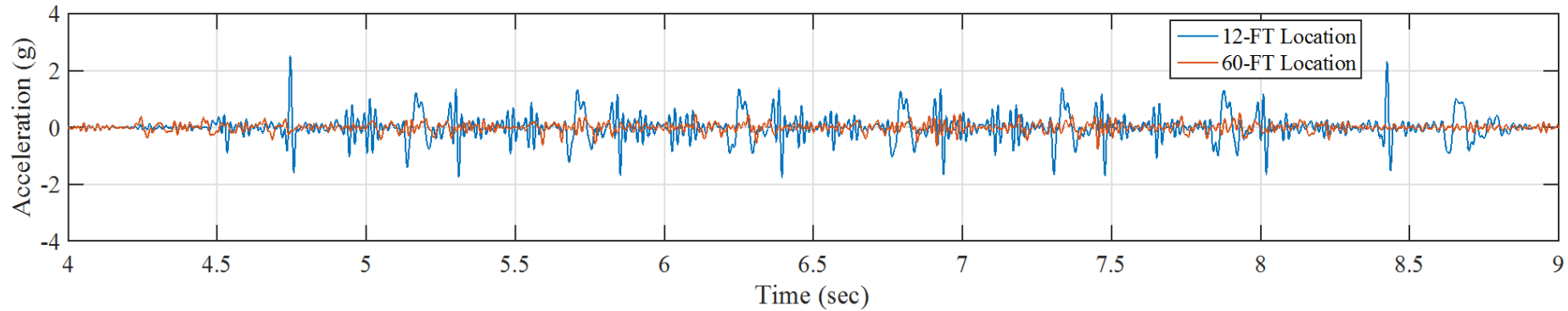
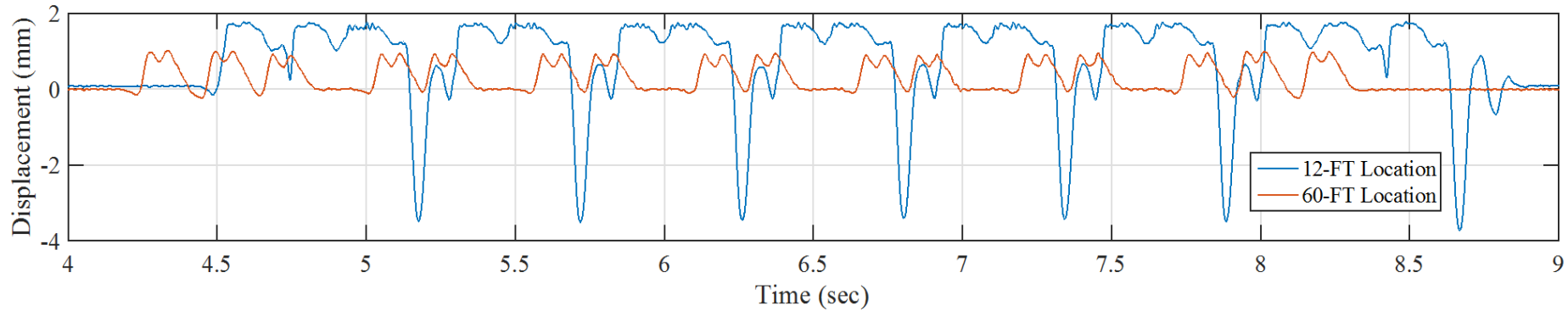
Upland St. Track 3 – ACELA Train



November 2012 – LVDT 1 Data Madison St. Track 2 – ACELA Train



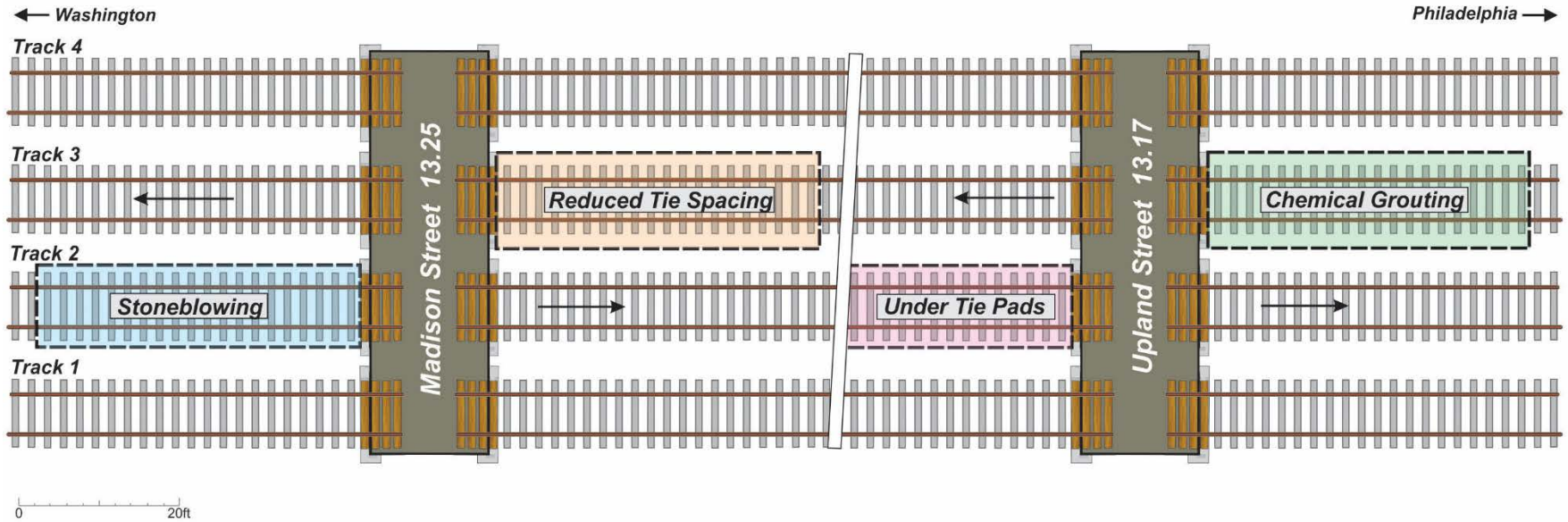
June 2013 – LVDT 1 Data Madison St. Track 2 – ACELA Train



Summary of Observations from Field Instrumentation

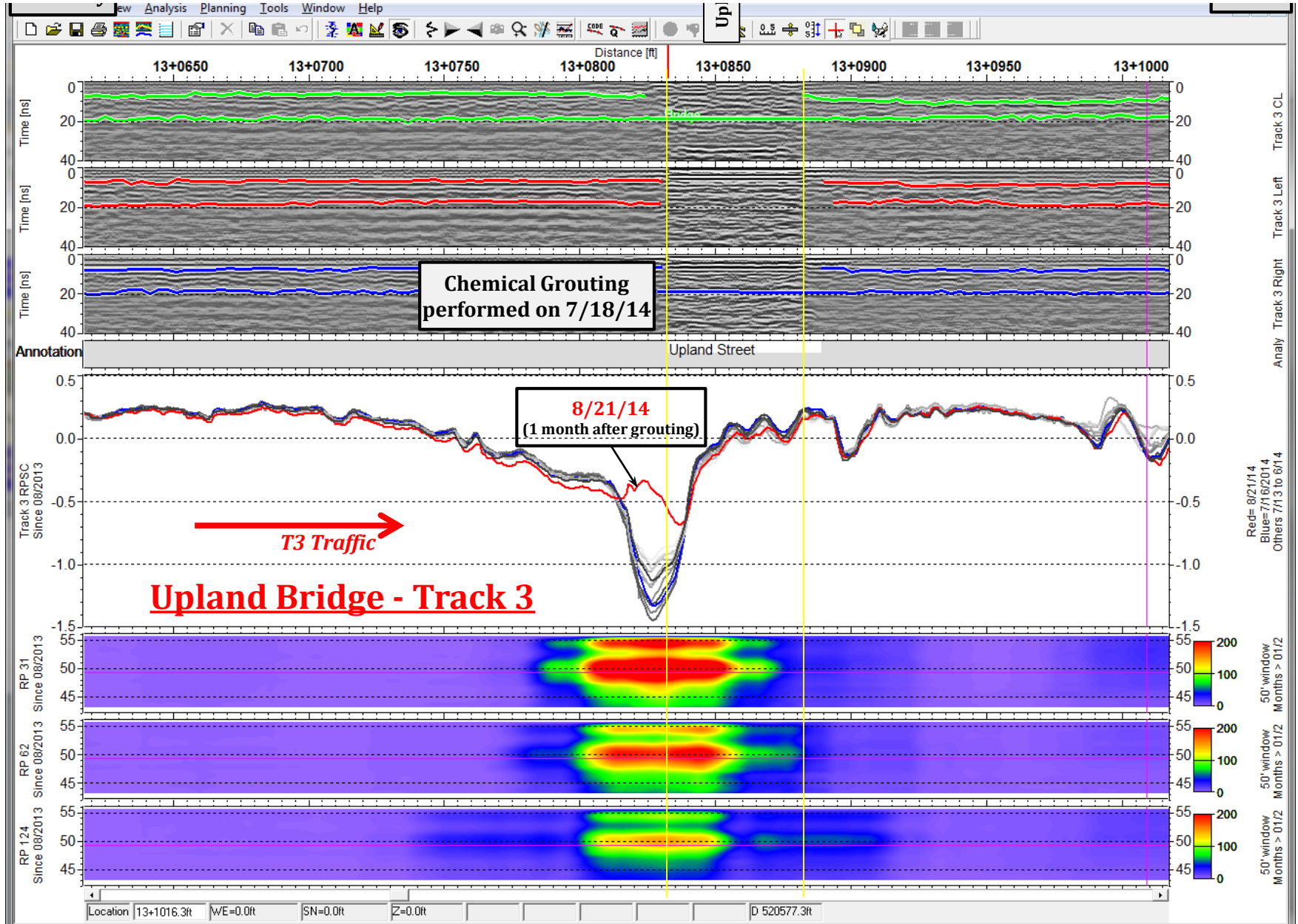
- ✓ **MDD and strain gauge instrumentation technologies** were successfully employed to measure deformations of ballasted track substructure layers, wheel loads and tie reactions under high(er) speed passenger traffic at AMTRAK NEC in the US
- ✓ **Most of the deformations** at the instrumented bridge approaches at the AMTRAK NEC appear to be occurring in the **ballast layer**
- ✓ **“Near-surface” remedial measures** are recommended to mitigate the differential movement problem by arresting excessive ballast reorientation and movement

Remedial Measures & Implementation Details

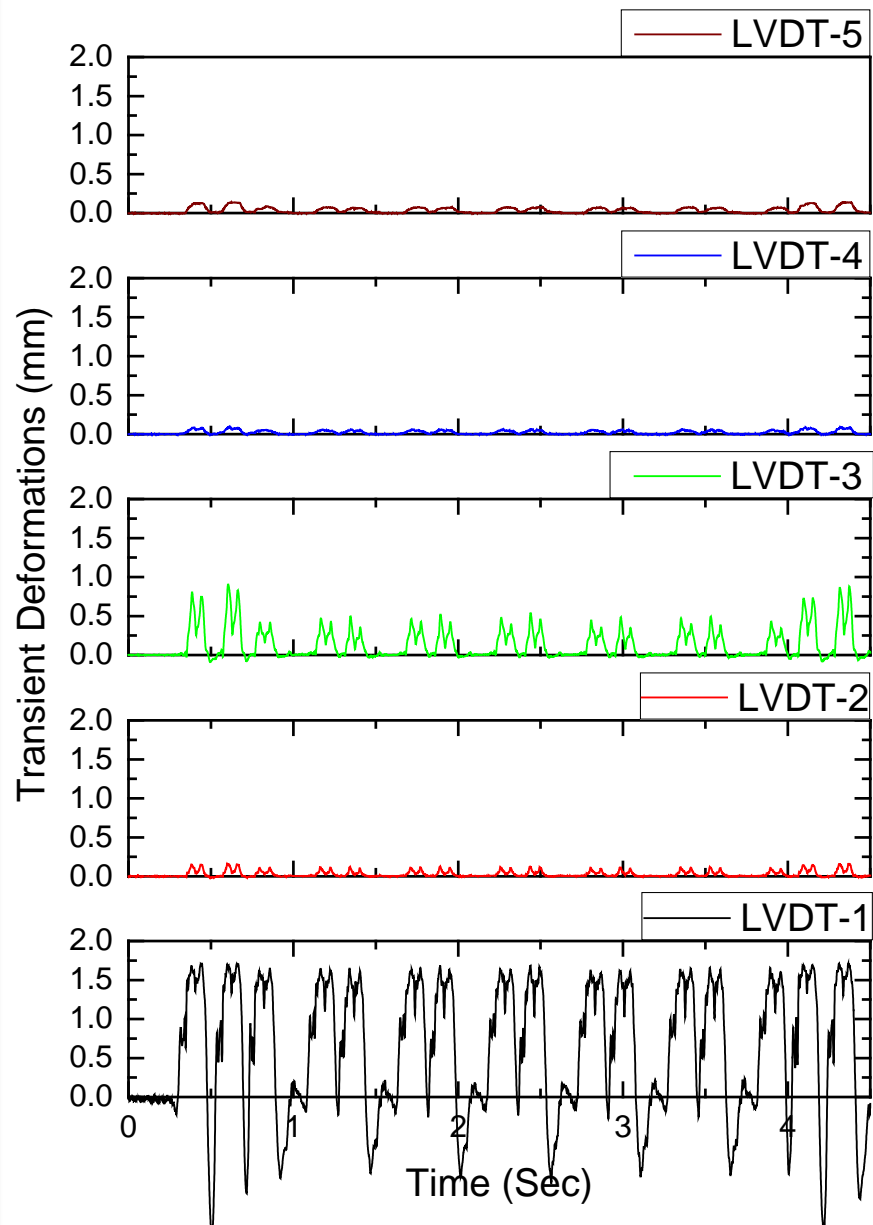


Bridge / Approach	Track	Remedial Measure	Approximate Length of Implementation
Upland / North	3	Chemical Grouting	40 ft.
Upland / South	2	Under-Tie Pads	60 ft.
Madison / North	3	Reduced Tie Spacing	60 ft.
Madison / South	2	Stone blowing	Based on Void Measurements

Chemical Grouting of Ballast: July 17, 2014

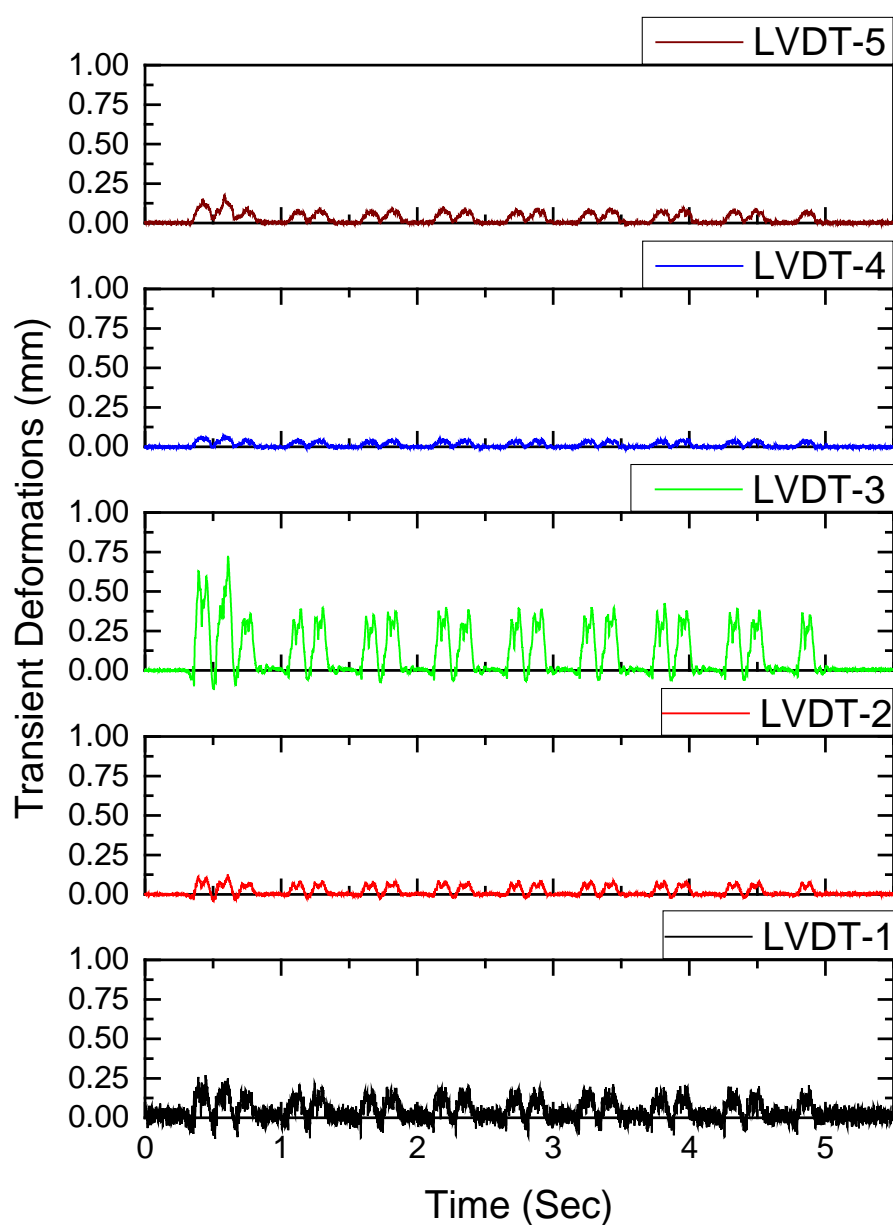


Upland 15 ft. (7 Aug 2012)



After Instrumentation in 2012

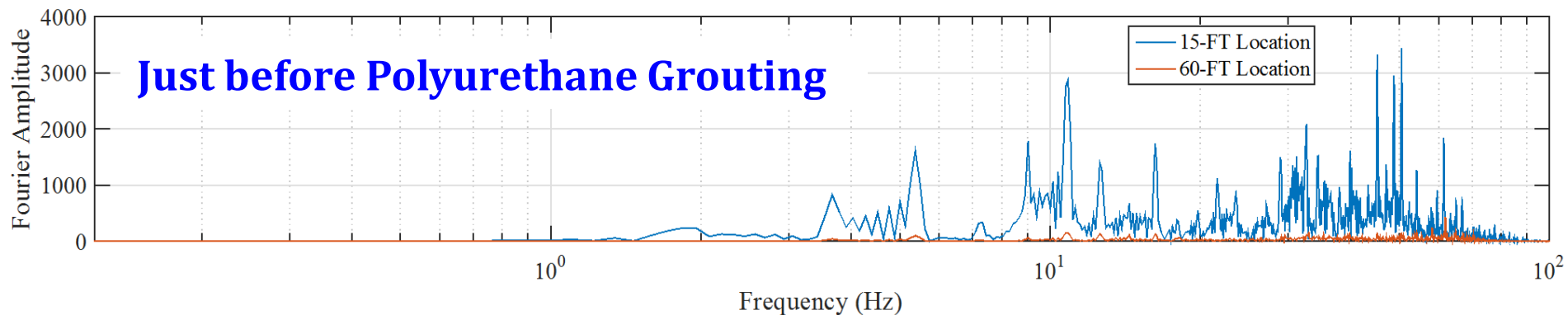
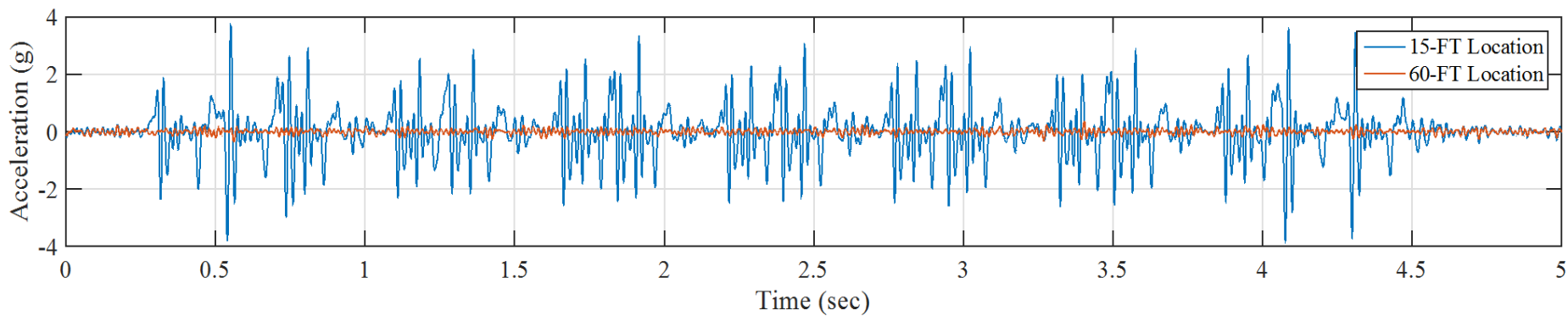
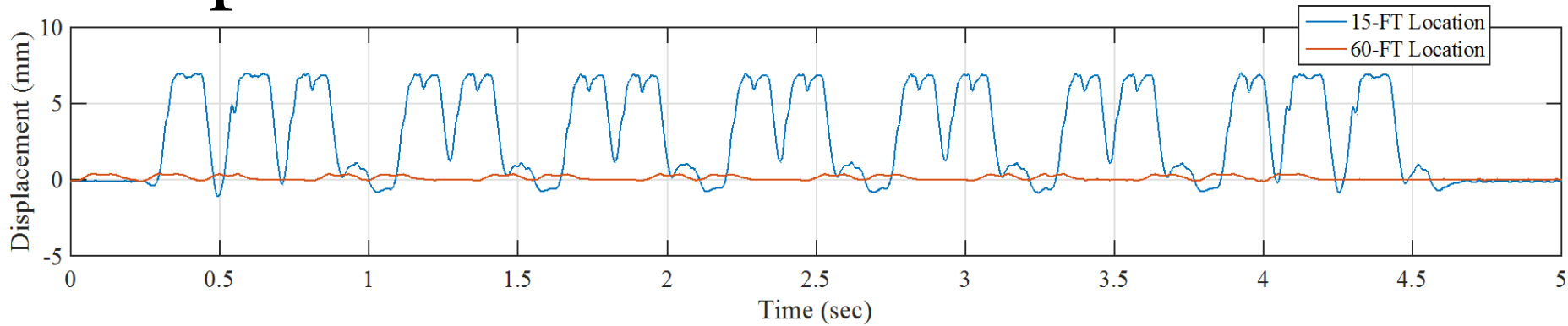
Upland 15 ft. (22 July 2014)



Post-Chemical Grouting

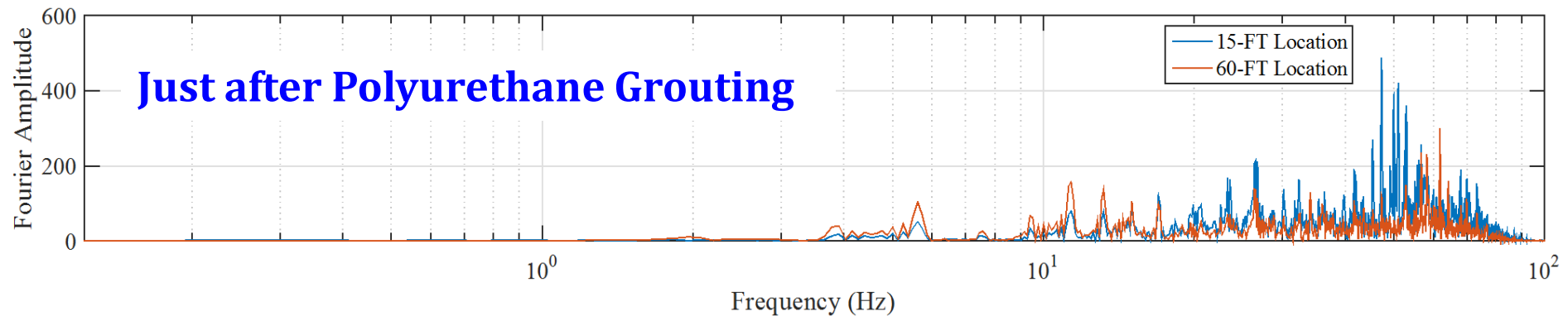
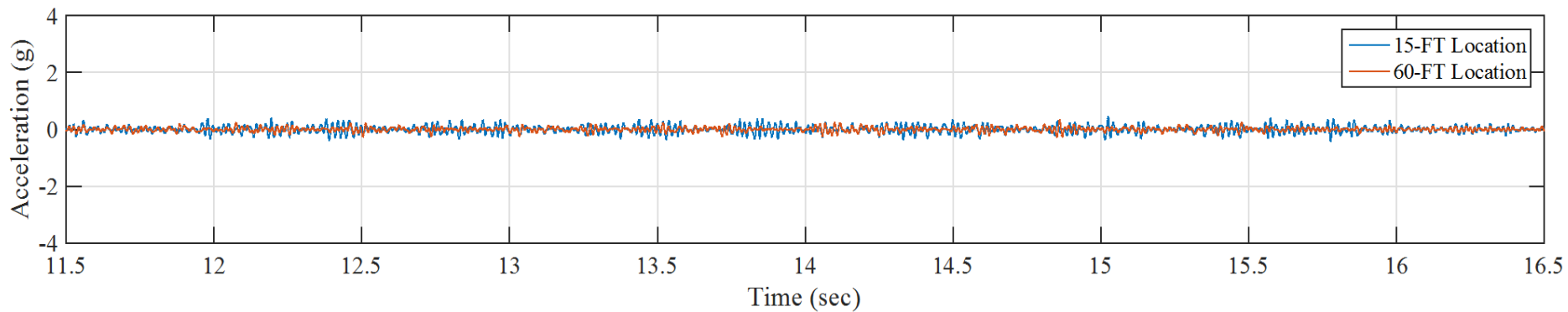
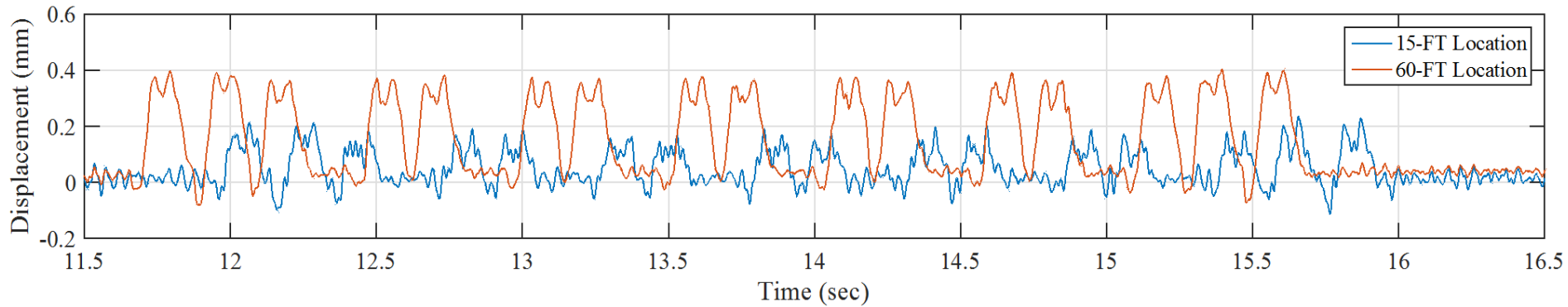
July 1, 2014 – LVDT 1 data

Upland St. Track 3 – ACELA Train



July 22, 2014 – LVDT 1 data

Upland St. Track 3 – ACELA Train



Chemical Grouting of Ballast: Oct. 23, 2014

Porter Street - Track 2- South Approach

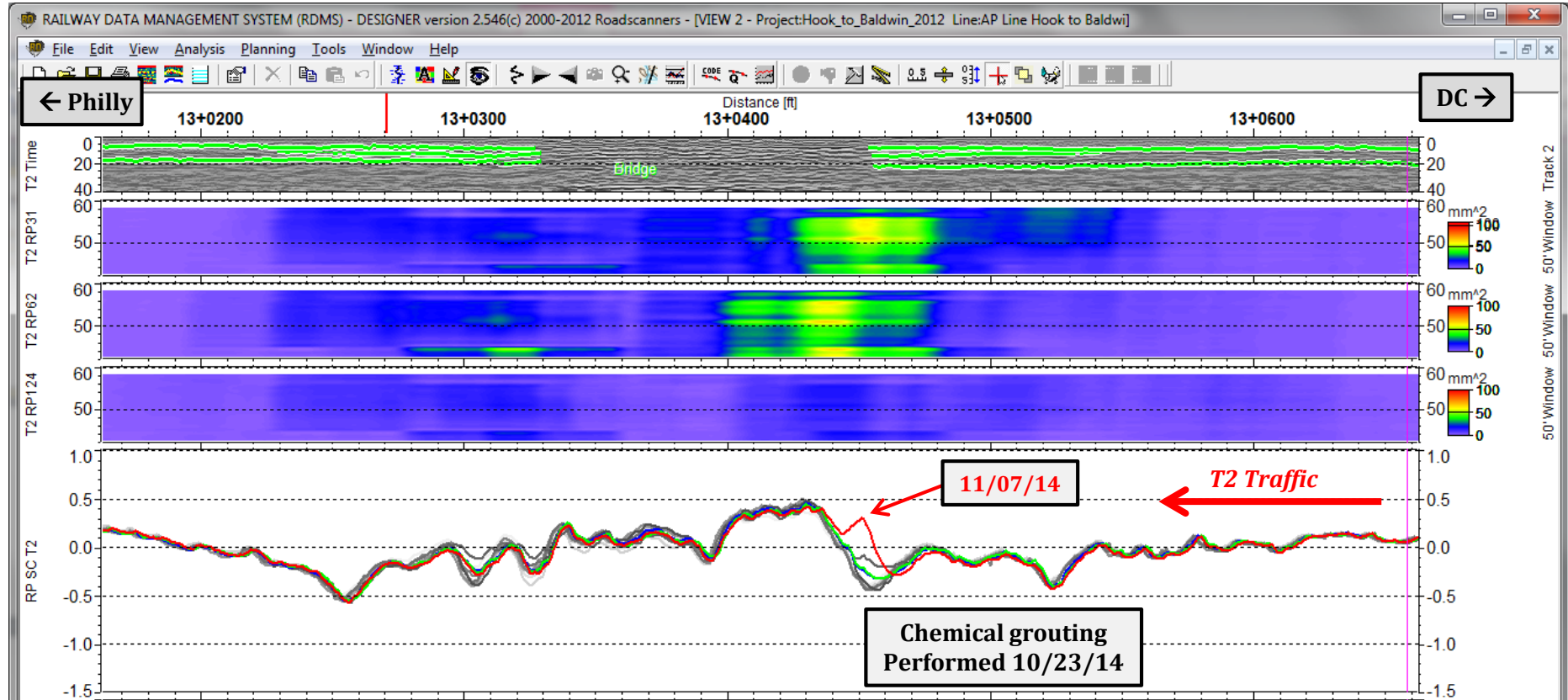


- ✓ The procedure was improved considerably
- ✓ New ballast and increased amounts of void space in the matrix led to increased grout consumption

**7-8 Ties were Successfully
Polyurethane Grouted**

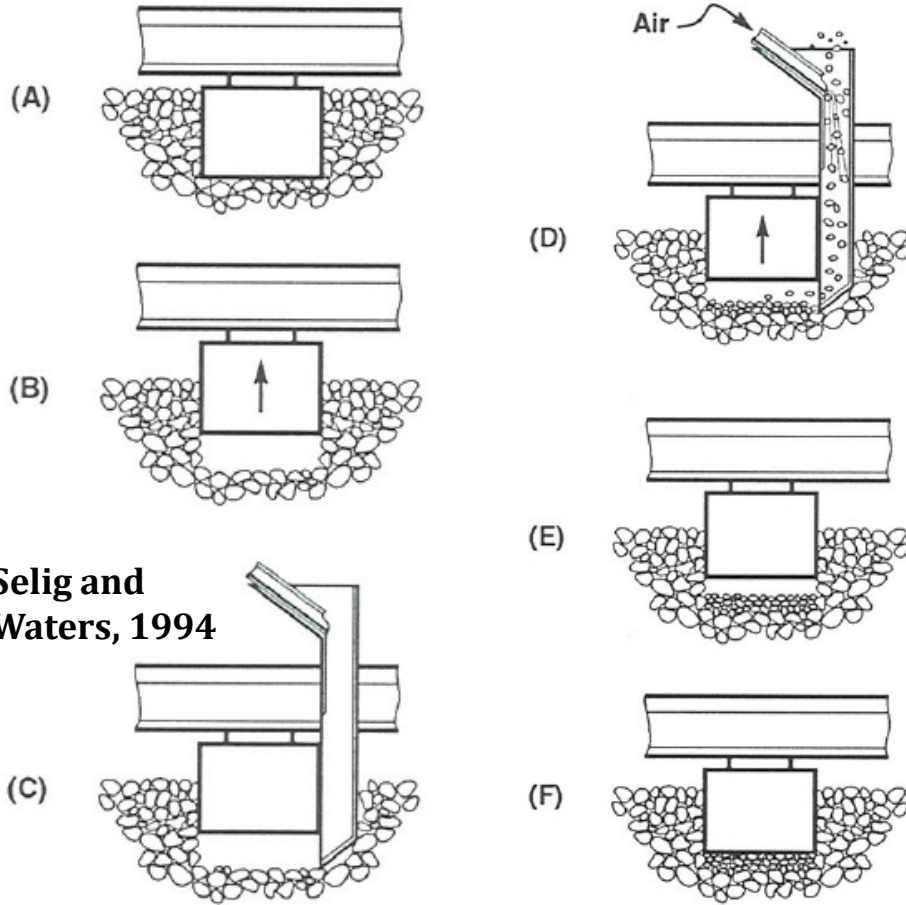


Morton & Porter Streets- Track 2



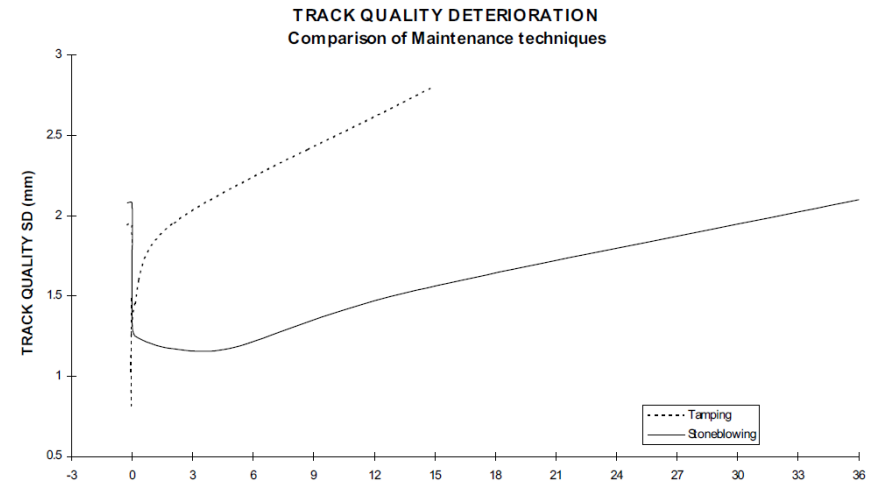
The profile of chemical grouting as of 11/07/2014

Stone Blowing



Selig and Waters, 1994

A	Initial condition
B	Ties raised
C	Stoneblowing tubes driven next to tie
D	Compressed air used to blow stones
E	Stoneblowing tubes withdrawn
F	Ties lowered to rest on freshly inserted stone



McMichael and McNaughton, 2003

Stone Blowing: October 27, 2014



Madison St. Bridge - Track 2

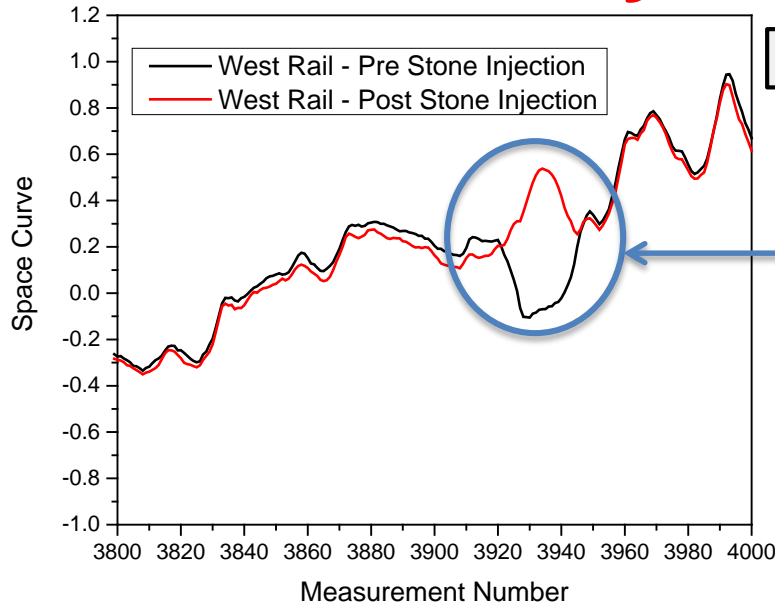
Injection Chute with Air
Connection Mounted on Top
of the Injector Tube

Inject known quantity
of ½-in. stone

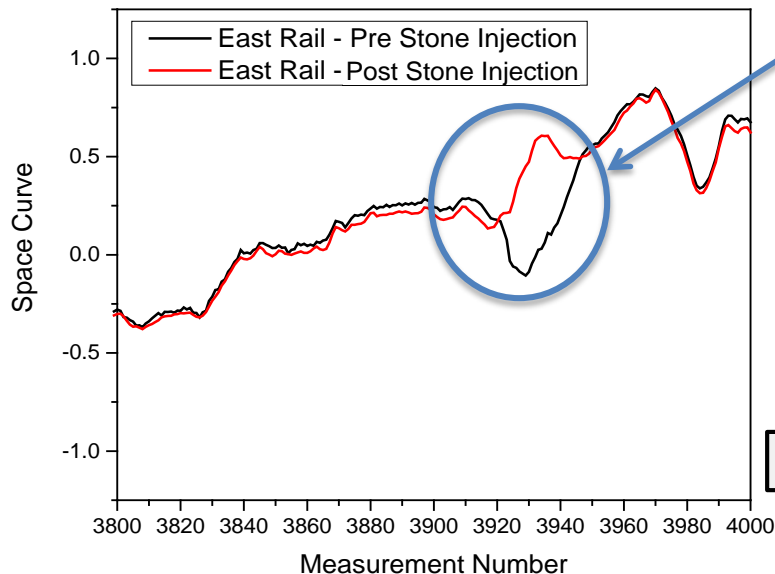
1 lb. stone for 1/32" lift



Track Geometry Data: Post Stone Blowing

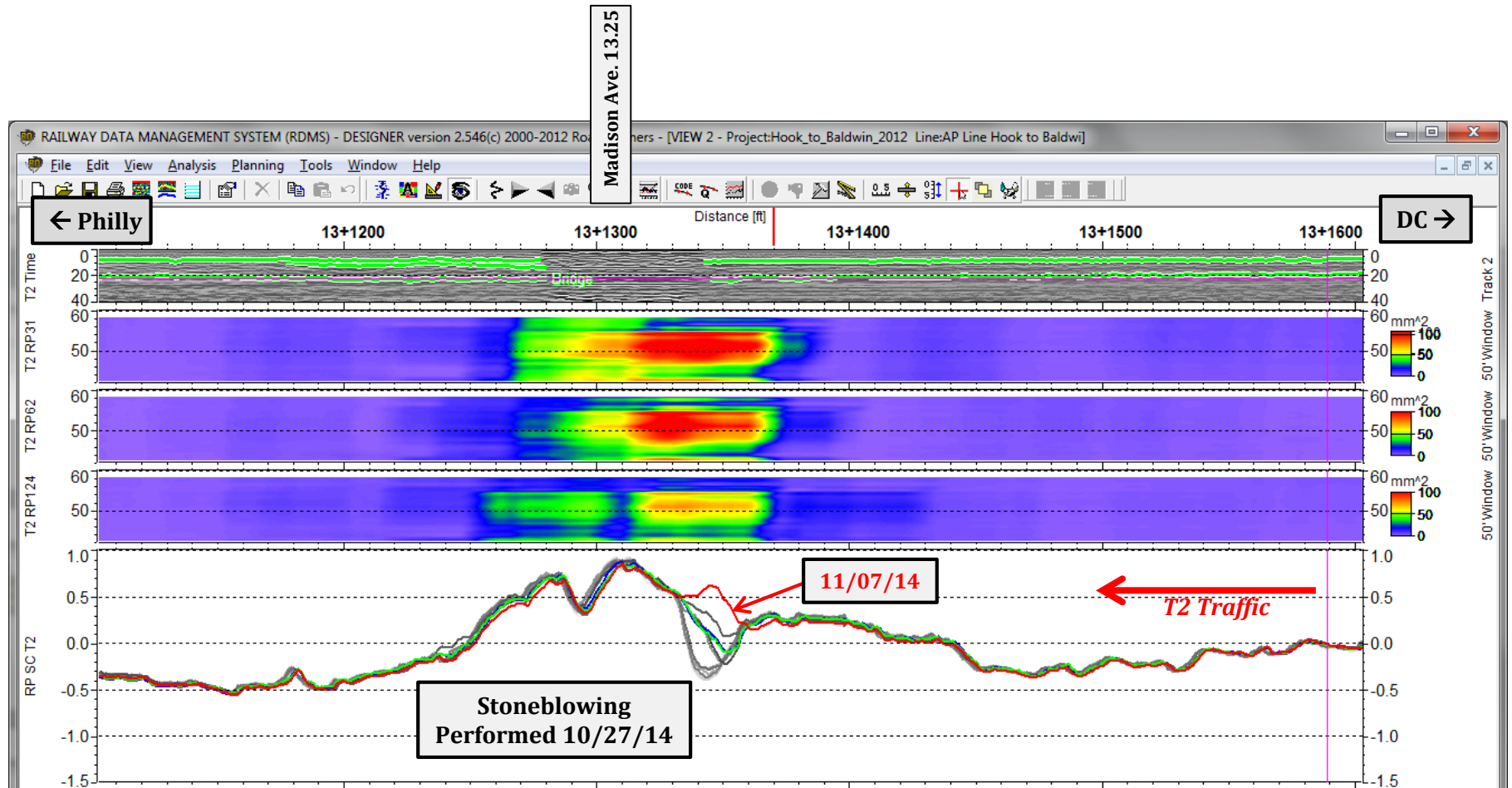


Track geometry data indicates the designed over lift has not dissipated as of 11/07/2014



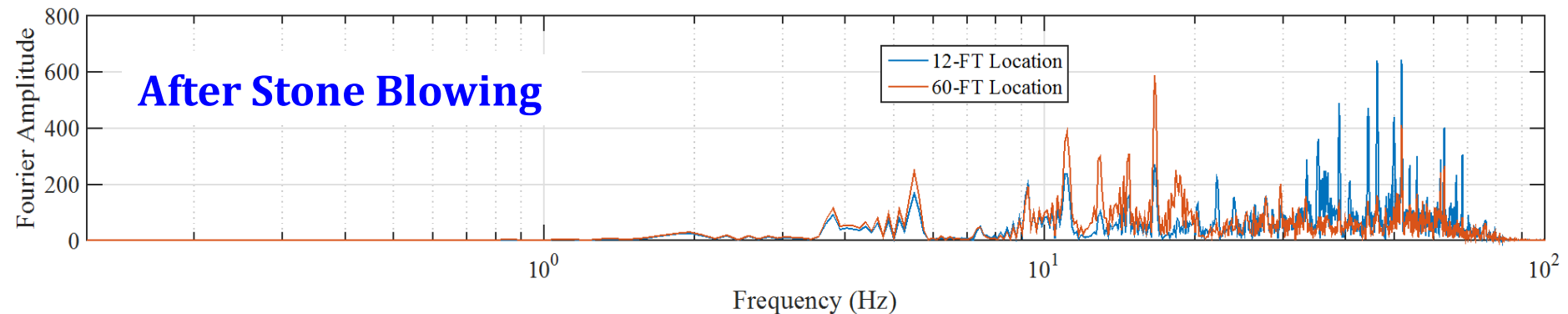
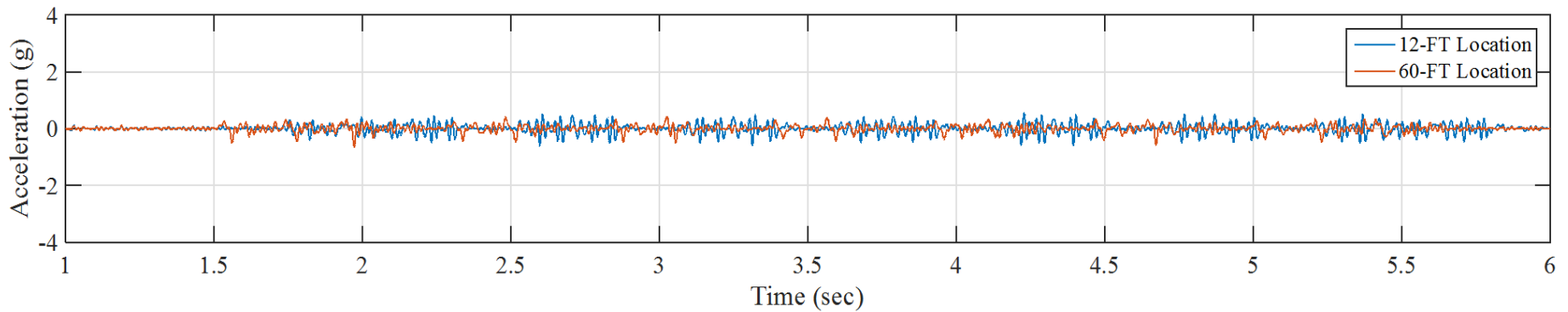
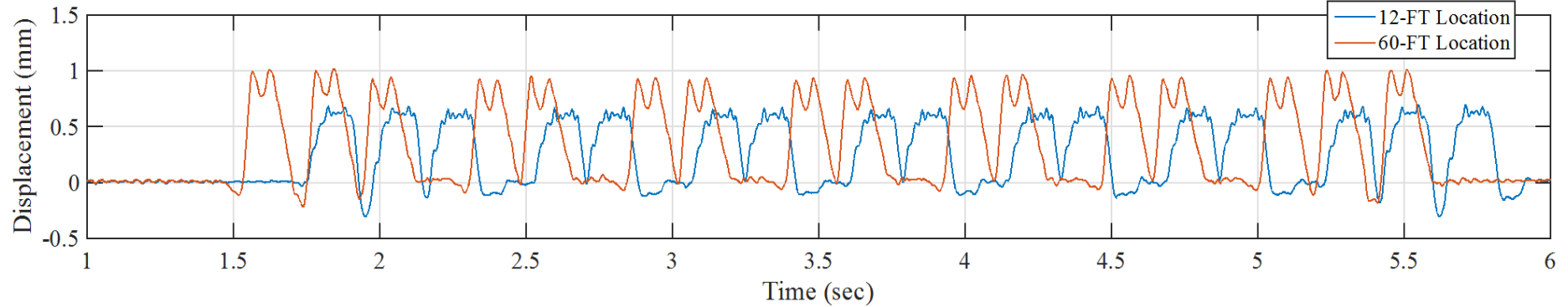
- Implementation: 27 October 2014
- Venue: Madison Street, Track 2 South Approach
- 12 ties successfully stone injected to provide better seating

Madison St. Bridge - Track 2

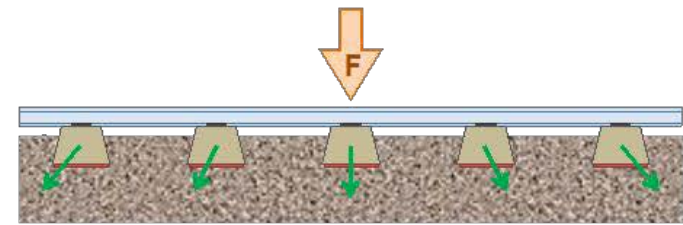
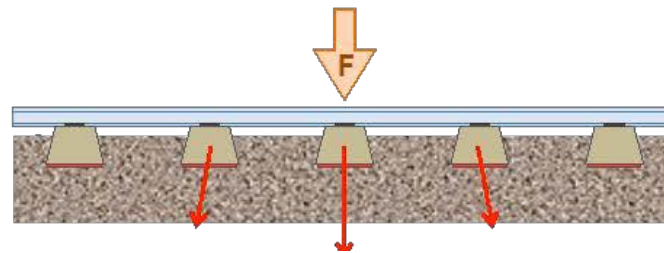
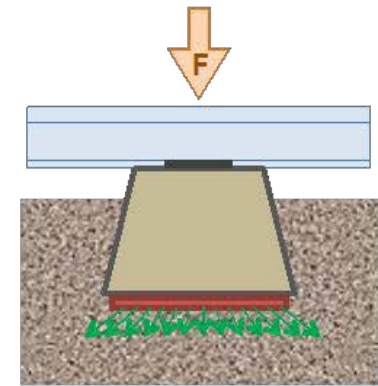
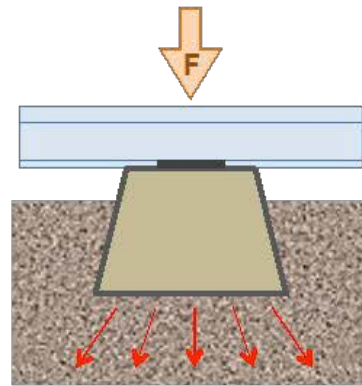
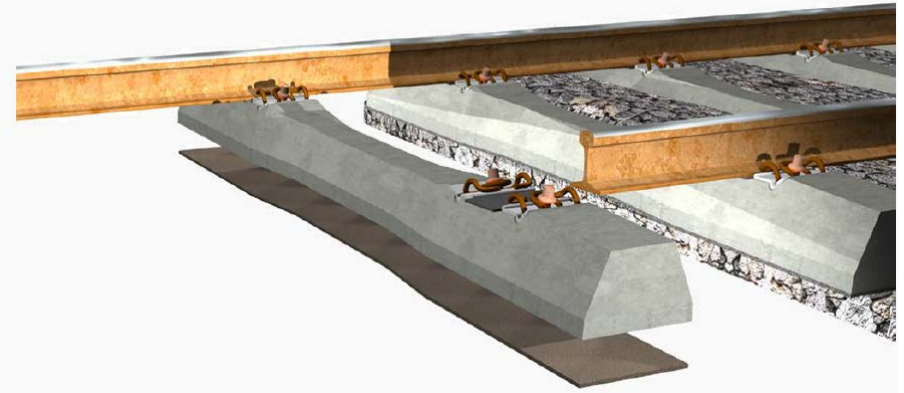


The profile of stone injection (12 ties) as of 11/07/2014

January 2015 – LVDT 1 Data Madison St. Track 2 – ACELA Train



Installation of Under Tie Pads



Without Under-Tie Pads

With Under-Tie Pads

Figure Source:

<http://cdm-novitec.com/onewebmedia/Brochures/CDM-UTP%20Brochure%20-%20020713.pdf>

TC202 Workshop on Railroad Geotechnics, ECSMGE, Edinburgh – Sept. 13, 2015

Under Tie Pad Track Panel Installation: Aug. 28-29, 2014

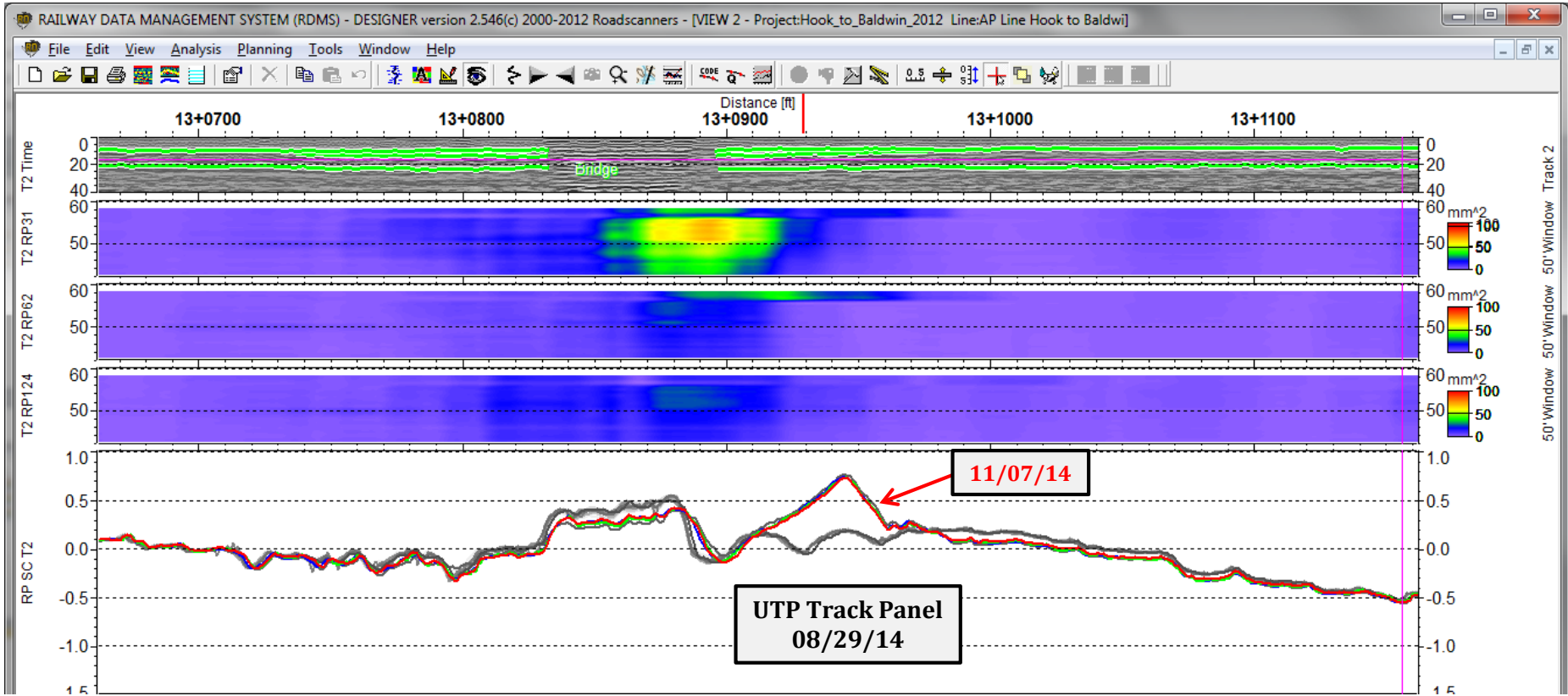
30 Tie Track Panel with Installed UTPs under New Ties



Track 2 – Upland St. Bridge



Track 2 – Upland St. Bridge



The profile of UTP track panel (30 ties) as of 11/07/2014

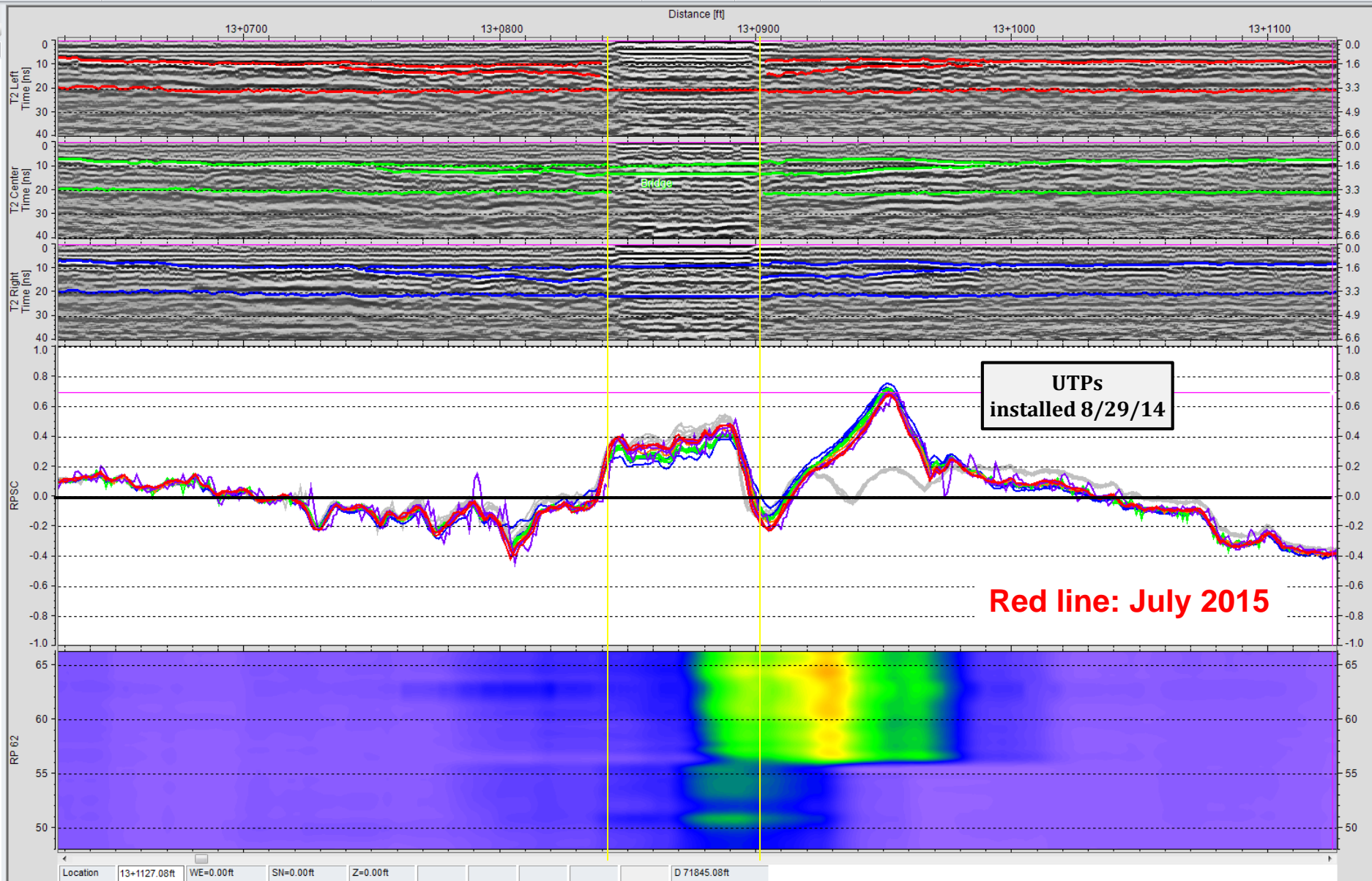
Latest Performance Data

← Philly

Upland St.- Track 2

← T2 Traffic

DC →



Under Tie Pad Track Panel Instrumentation

August 2015

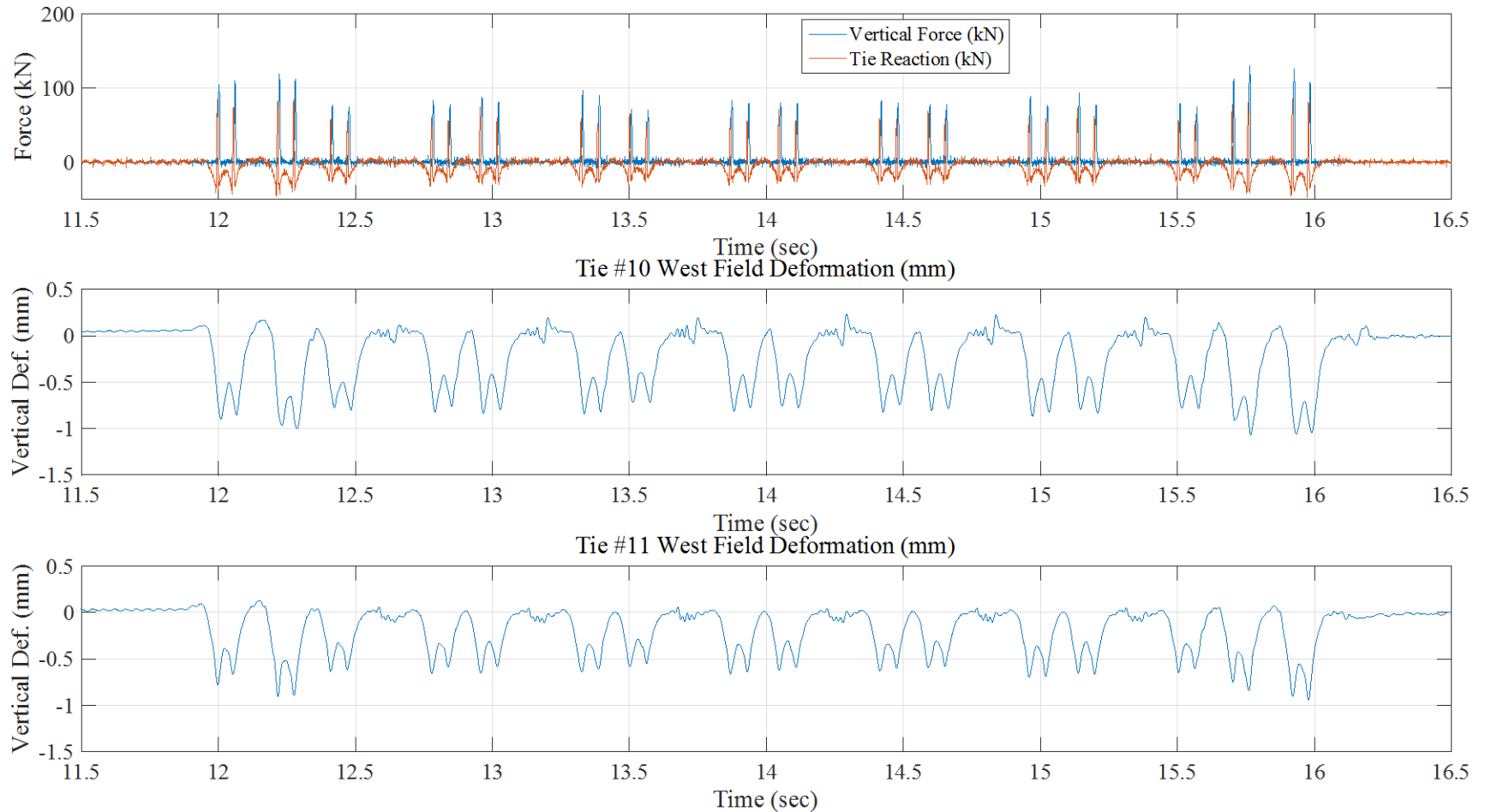


Strain Gauge Tie #10
Channel #10

Accelerometer Tie #10
West Field Site

Strain Gauge Tie #11
Channel #9

Under Tie Pad Track Panel Performance



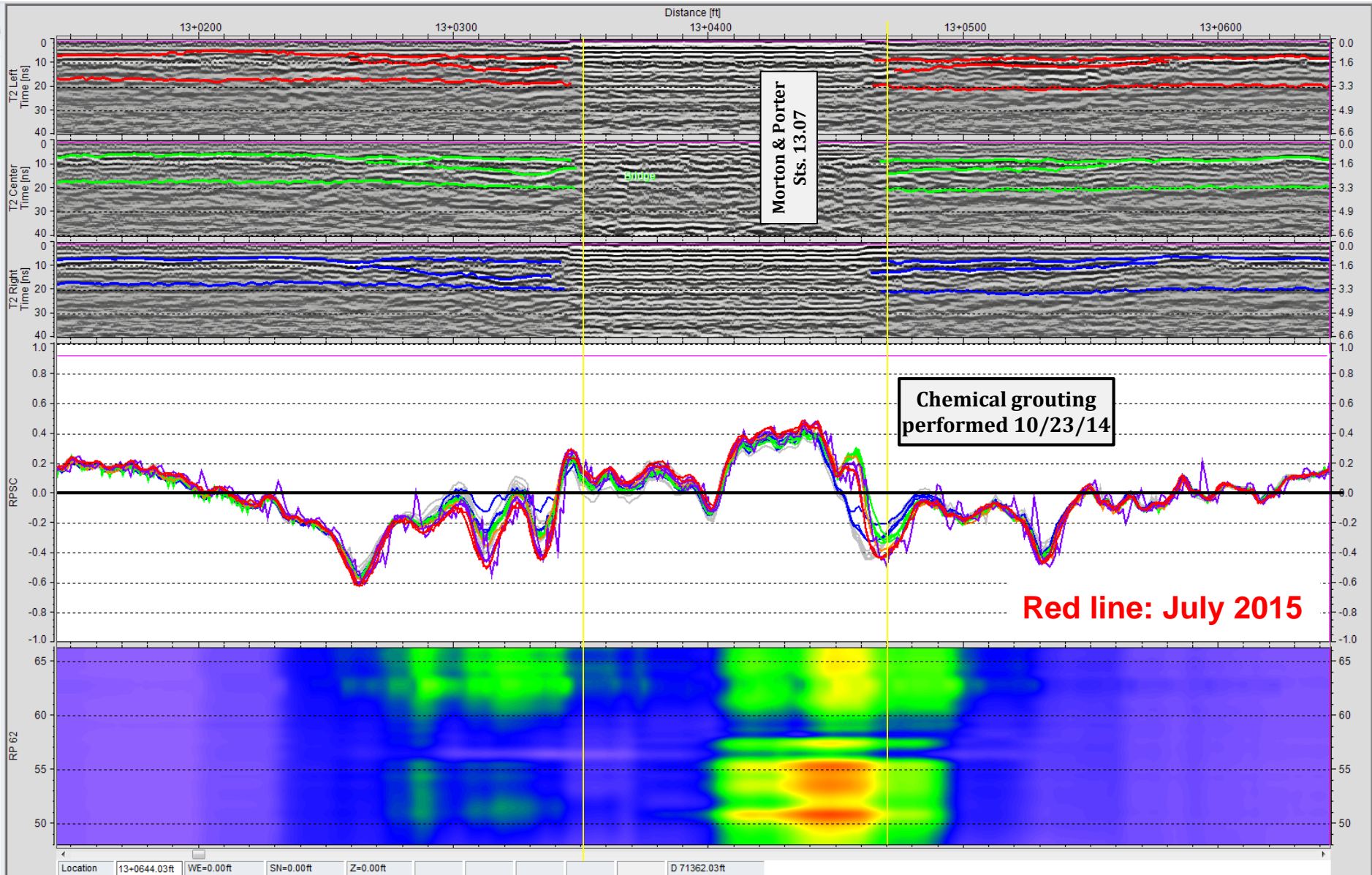
Deformations are less than or slightly over 1 mm. There is no excessive heaving or negative displacement.

← Philly

Morton & Porter Sts.- Track 2

← T2 Traffic

DC →

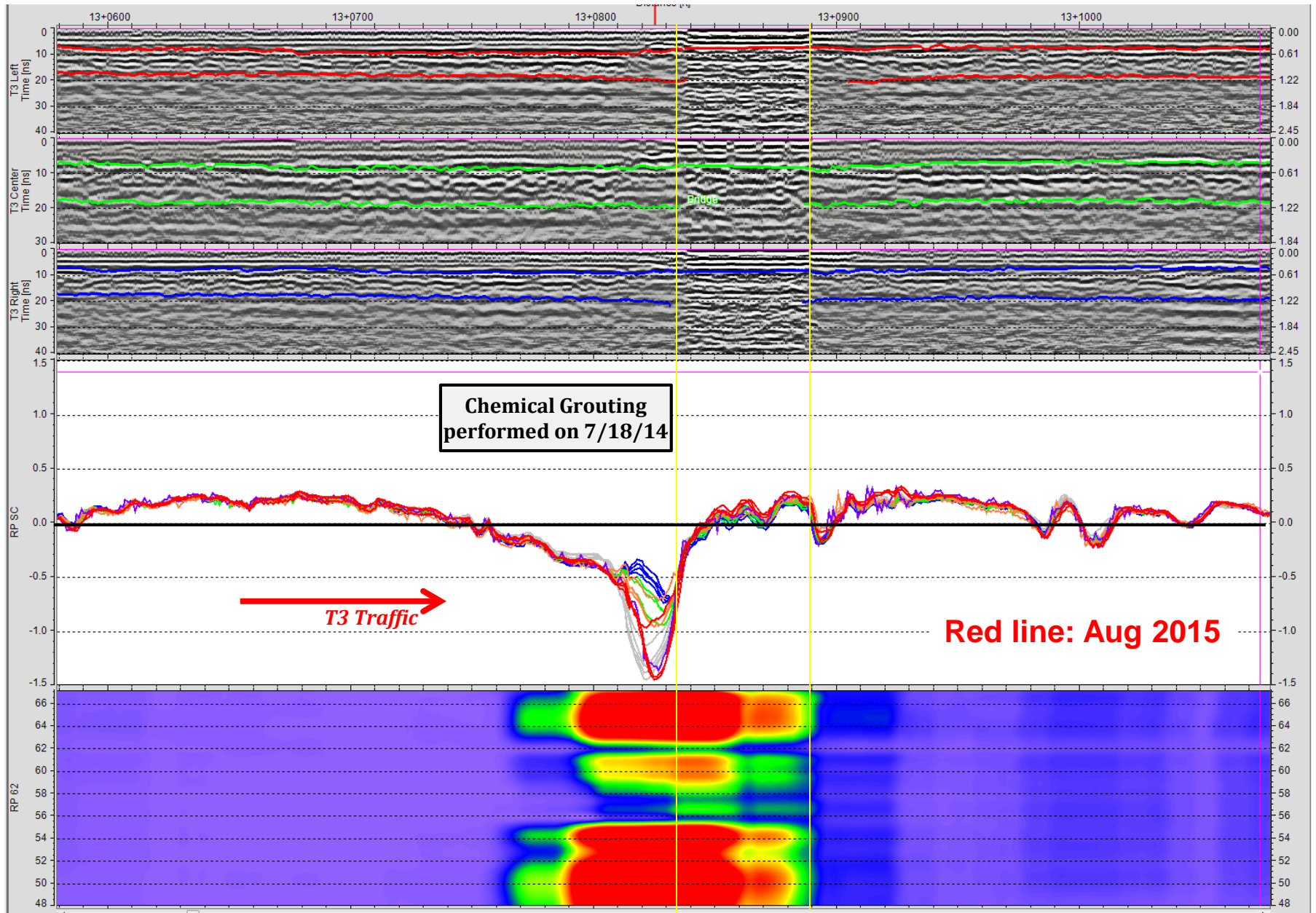


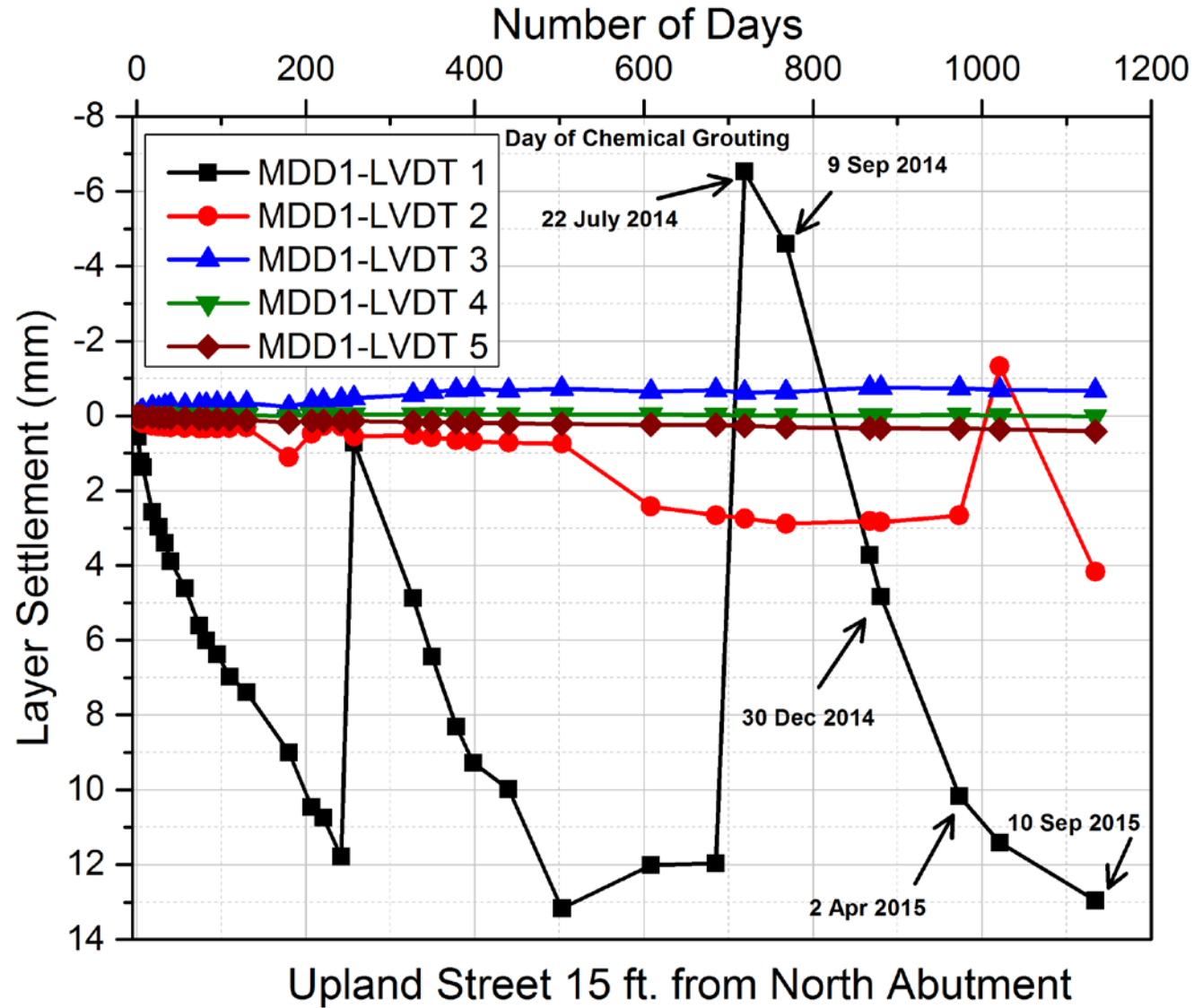
Upland Bridge - Track 3

← Philly

DC →

Upland St. 13.17



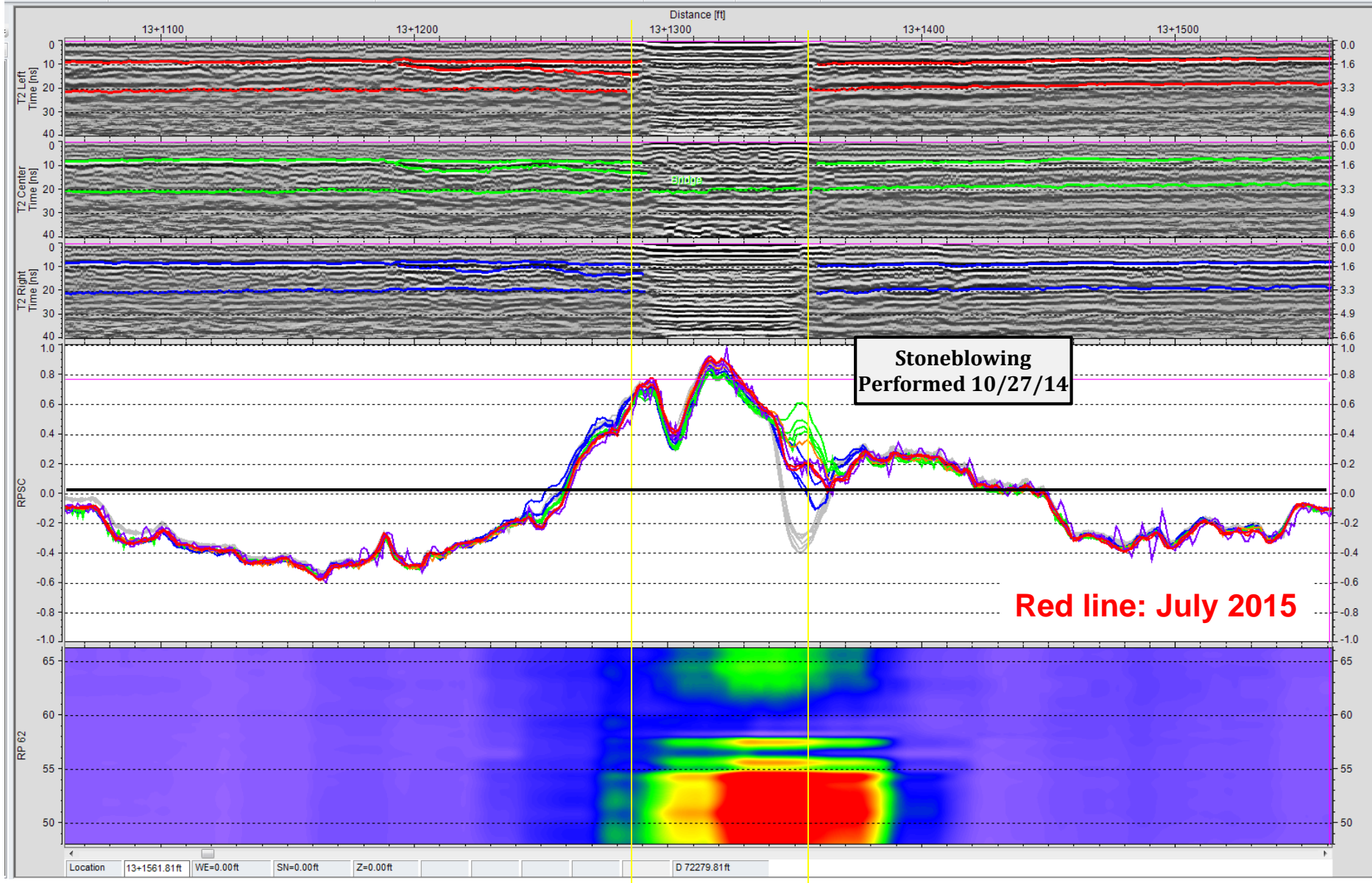


← Philly

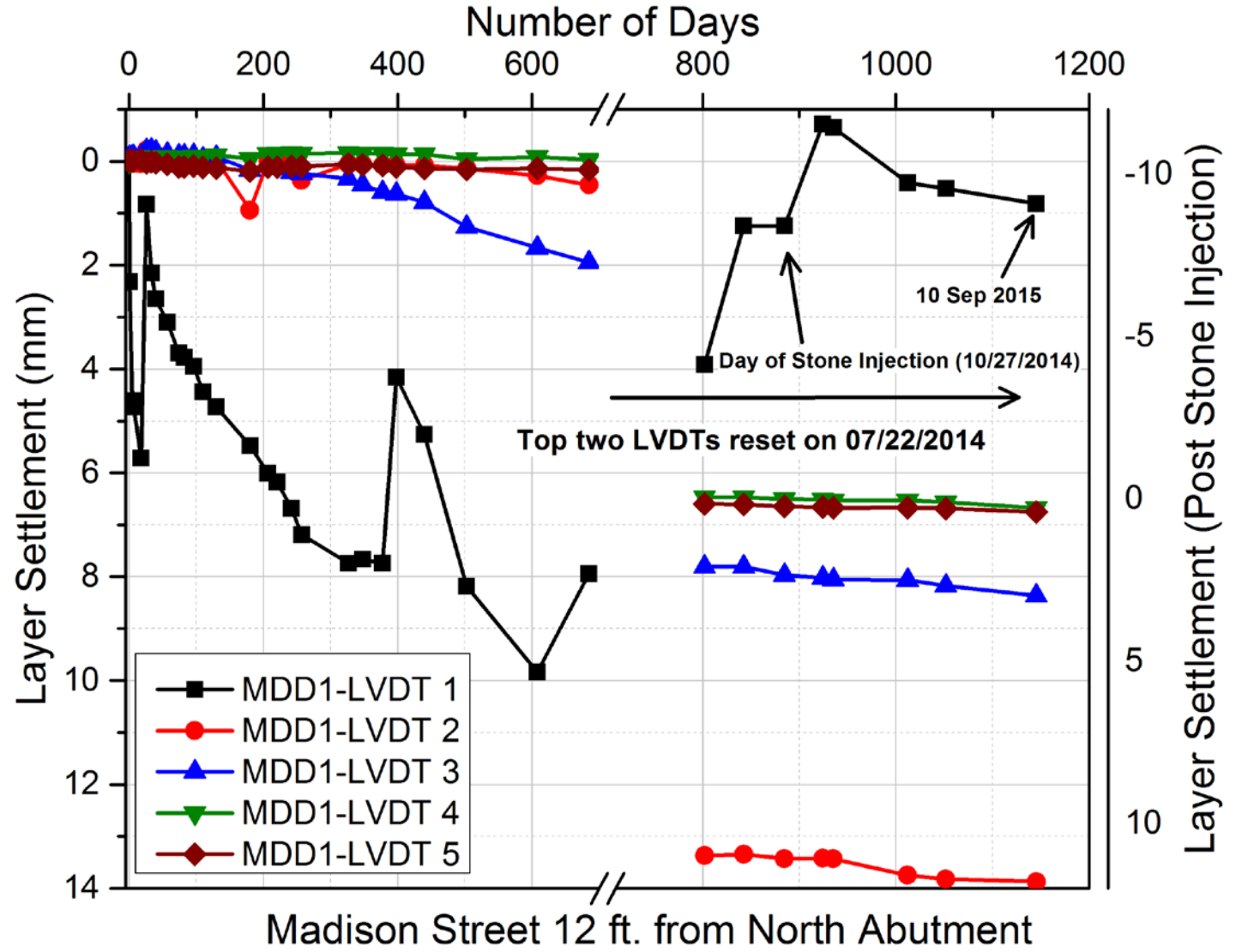
Madison St. Bridge - Track 2

← T2 Traffic

DC →



TC202 Workshop on Railroad Geotechnics, ECSMGE, Edinburgh – Sept. 13, 2015



Acknowledgments

- ✓ UIUC research team – Debakanta Mishra, Tim Stark, Jim Hyslip, Mike Wnek, Huseyin Boler, Hasan Kazmee, Liang Chow



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 - ✓ Mike Tomas
 - ✓ Steve Chrismer
 - ✓ Mike Tirosino
 - ✓ David Staplin



- ✓ Norfolk Southern
 - ✓ Brad Kerchof



Thank you!
Questions?

